

GEI Consultants

Regulation Compliance Report

Weston Disposal Site No. 3 Ash Landfill Run-on and Run-off Control Plan

Town of Knowlton, Marathon County, Wisconsin

Submitted to:
WEC Energy Group – Business Services
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Submitted by:
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October 2026
Project No.2103691



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Standard Limitations and Disclaimer

This report was prepared exclusively for the use of WEC ENERGY GROUP. The findings and conclusions, if any provided by GEI in this report, are based solely on the information reported to GEI as of the date of this report. Future investigations or additional information not provided to GEI at the time of this report may result in modification of this report. GEI's scope of work did not include verifying the completeness or accuracy of information provided by others. Accordingly, GEI shall not be liable for any damages, costs, or other consequences resulting from reliance on such information if it is later determined to be inaccurate or incomplete. GEI's professional services for this project have been performed in a manner consistent with that degree of skill and care ordinarily exercised by members of the same profession currently practicing in the same locality, performing similar services under similar conditions. GEI makes no other representations and no warranties, express or implied.

Certification

This plan was completed under the direction of John, M. Trast, P.E. I am a licensed professional engineer in the State of Wisconsin in accordance with the requirements of ch. A-E 4, Wisconsin Administrative Code; that this document has been prepared in accordance with the Rules of Professional Conduct in ch. A-E 8, Wisconsin Administrative Code; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in 40 CFR Part 257 Subpart D.



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Record of Revisions

Identification	Date	Description of Issued and/or Revision
Revision 0	October 2016	Initial submittal.
Revision 1	October 2021	This plan was updated in accordance with § 257.81(c)(4) which required the owner or operator of the CCR unit to prepare periodic run-on and run-off control system plans every five years. Updated the existing site conditions and engineering calculations.
Revision 2	October 2026	This plan was updated in accordance with § 257.81(c)(4) which required the owner or operator of the CCR unit to prepare periodic run-on and run-off control system plans every five years. Updated the existing site conditions and engineering calculations.

1. Introduction

WEC Energy Group (WEC) owns and operates the Weston Disposal Site No. 3 Ash Landfill, located in the E 1/2 of the NW 1/4 and W 1/2 of the NE 1/4, Section 23, Township 26 North, Range 7 East, Town of Knowlton, Marathon County, Wisconsin. The WEC Weston Disposal Site No. 3 Ash Landfill is regulated as an industrial waste landfill by the Wisconsin Department of Natural Resources (WDNR) under the provisions of Chapter 289 Wisconsin Statutes, and all applicable requirements of Chapters NR 500 of the Wisconsin Administrative Code. The design, construction, operation, closure, and post-closure care requirements are specified in the WDNR conditionally approved Plan of Operations, License No. 3067, FID No. 737025120. Cells 1 and 2 were constructed in 2015, including installation of a leachate force main, storage tanks, and load-out system in late December. WDNR approved construction on April 22, 2016. Cell 2 began accepting waste on June 27, 2016, followed by Cell 1 on August 27, 2021. Filling has occurred intermittently, and approximately 5.4 acres of exterior slopes have received final cover. Cell 3 was constructed in 2026 but had not received WDNR approval for filling as of October 2026.

In addition to the state regulations, the landfill is also required to comply with 40 CFR Part 257 Subpart D – *Standards for Disposal of Coal Combustion Residuals in Landfills and Surface Impoundments.* Weston Disposal Site No. 3 Ash Landfill, Cells 1, 2, and newly constructed Cell 3, are defined as CCR units and existing CCR landfills in accordance with § 257.53 since construction commenced prior to October 14, 2015. Future landfill cells are permitted by the WDNR in the approved Plan of Operation and defined as lateral expansions under § 257.53 when constructed.

This report presents the five-year update to the run-on and run-off control plan for Weston Disposal Site No. 3, Cells 1–3, under § 257.81. It describes the control systems and provides supporting engineering calculations addressing the applicable design and construction requirements of § 257.81(c)(1).

This run-off and run-on system control plan includes the following sections:

- Section 1 – Introduction
- Section 2 – Storm and Stormwater Volume Determination
- Section 3 – Run-on Control System
- Section 4 – Run-off Control System
- Section 5 – Conclusion
- Section 6 – References

2. Storm and Stormwater Volume Determination

Section 257.81, *Run-on and run-off controls for CCR landfills*, requires owners or operators of existing CCR landfills, new CCR landfills, and lateral expansions to design, construct, operate, and maintain run-on and run-off control systems. Run-on controls must prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm. Run-off controls must collect and control at least the water volume generated by that storm on the active portion of the CCR unit.

Cells 1, 2, and 3 occupy approximately 6.6, 8.6, and 4.2 acres, respectively. The active filling area is approximately 14.1 acres, and approximately 5.4 acres have received final cover. Precipitation within the active filling area is contained within the permitted limits of waste and managed as leachate. Precipitation on the final cover is directed off the landfill as noncontact water. Drawing C-1 – Weston Disposal Site No. 3 Cells 1 – 3 (Appendix A) shows the existing waste grades for Cells 1–3 as of June 19, 2026.

The estimated rainfall depth for the 24-hour, 25-year storm at Weston Disposal Site No. 3 is 4.47 inches, based on NOAA Atlas 14, Volume 8, Version 2. Supporting rainfall and run-off volume calculations are provided in Appendix B: NOAA 14, Vol. 8 Rainfall Analysis and Run-off Volume.

Table 2-1 summarizes the design storm, rainfall depth, active lined area, and minimum run-off volume to be managed within Cells 1–3.

Table 2-1. Design Storm Rainfall and Run-off Volume

Storm Recurrence Interval	Rainfall Depth (inches)	Active Landfill Lined Area (acres)	Run-off Volume (acre-ft)
24-hour, 25-year	4.47	14.1	5.25

3. Run-on Control System

Section 257.81(a)(1) requires run-on controls to prevent flow onto the active portions of the CCR unit during the peak discharge from a 24-hour, 25-year storm. The federal rule defines “Run-on” as “any rainwater, leachate, or other liquid that drains over land onto any part of a CCR landfill.”

Permanent perimeter berms along the north, east, and south sides of the landfill divert stormwater away from the active landfill. Temporary intercell berms along the west sides of Cells 1 and 3 provide equivalent run-on protection.

Approximately 5.4 acres of the perimeter slopes of Cells 1 and 2 have received final cover. Stormwater from the final cover flows to a diversion berm ditch near the toe of the cover and discharges to Storm Water Basin No. 1, No. 2, or No. 3. Run-on controls are shown on Figure 1 in Appendix C – Stormwater Run-on Calculations.

HydroCAD 10.0 was used to evaluate whether the run-on controls serving the final-cover area can manage the 24-hour, 25-year storm without allowing noncontact water to enter the limits of waste. The modeled peak water levels in the North, Northeast, and Southeast Ditches are 0.97, 1.06, and 1.35 feet, respectively. The results indicate that the diversion berm ditches have sufficient capacity to prevent run-on during the design storm. Supporting calculations are provided in Appendix C – Stormwater Run-on Calculations.

The review of current topography and stormwater calculations indicates that the run-on control system meets current engineering standards and complies with § 257.81(a)(1).

4. Run-off Control System

Section 257.81(a)(2) requires run-off controls to collect and control at least the water volume generated on the active portion of the CCR unit by a 24-hour, 25-year storm. The federal rule defines “Run-off” as “any rainwater, leachate, or other liquid that drains overland from any part of a CCR landfill.”

Precipitation within the active landfill is managed as contact stormwater and treated as leachate under § 257.3-3. In Cells 1 and 2, containment ditches route contact stormwater to detention areas NWP1, NWP2, and SWP along the Cell 1-4A, Cell 1-3, and Cell 2-3 intercell berms, respectively. Water infiltrates from these areas into the leachate collection system. In the newly constructed Cell 3, contact water infiltrates into the leachate collection system and flows south to the collection sump. Contact water is managed in accordance with the landfill Plan of Operations.

HydroCAD 10.0 was used to evaluate whether the existing run-off controls for Cells 1–3 can manage the 24-hour, 25-year storm. Supporting calculations are provided in Appendix D: Stormwater Run-off Calculations.

Stormwater flows off the active slopes as sheet flow into temporary containment ditches along the landfill cell perimeter. The ditches are at least 2 feet deep, with a 3H:1V exterior slope and a 2H:1V interior slope. The exterior slope follows the top of the leachate collection system’s granular drainage layer; the interior slope is cut into the disposed CCR. At closure, the ditches will be filled with soil or CCR before placement of the final cover system. A cattle bridge or cattle guard at the active landfill entry provides a hydraulic break to prevent stormwater from flowing down the access road and escaping the site.

The model results indicate that the perimeter ditches can contain and convey run-off from the 24-hour, 25-year storm to the four detention areas. Modeled freeboard in NWP1, NWP2, SWP, and C3P is 1.67, 0.57, 5.19, and 9.63 feet, respectively. Each detention area can contain the design storm without overtopping. Supporting results are provided in Appendix D.

5. Conclusion

The Weston Disposal Site No. 3 Ash Landfill is regulated as an existing CCR landfill under 40 CFR Part 257 Subpart D. The rule required an initial plan addressing the applicable operating criteria by October 17, 2016, with updates at intervals not exceeding five years from the previous plan's completion date. The written plan must also be amended whenever changed conditions would substantially affect it. This revision is the five-year update to the October 2021 plan. The revised plan must be placed in the facility's operating record under § 257.105(g). The owner or operator must also comply with the recordkeeping, notification, and internet requirements of §§ 257.105(g), 257.106(g), and 257.107(g), respectively.

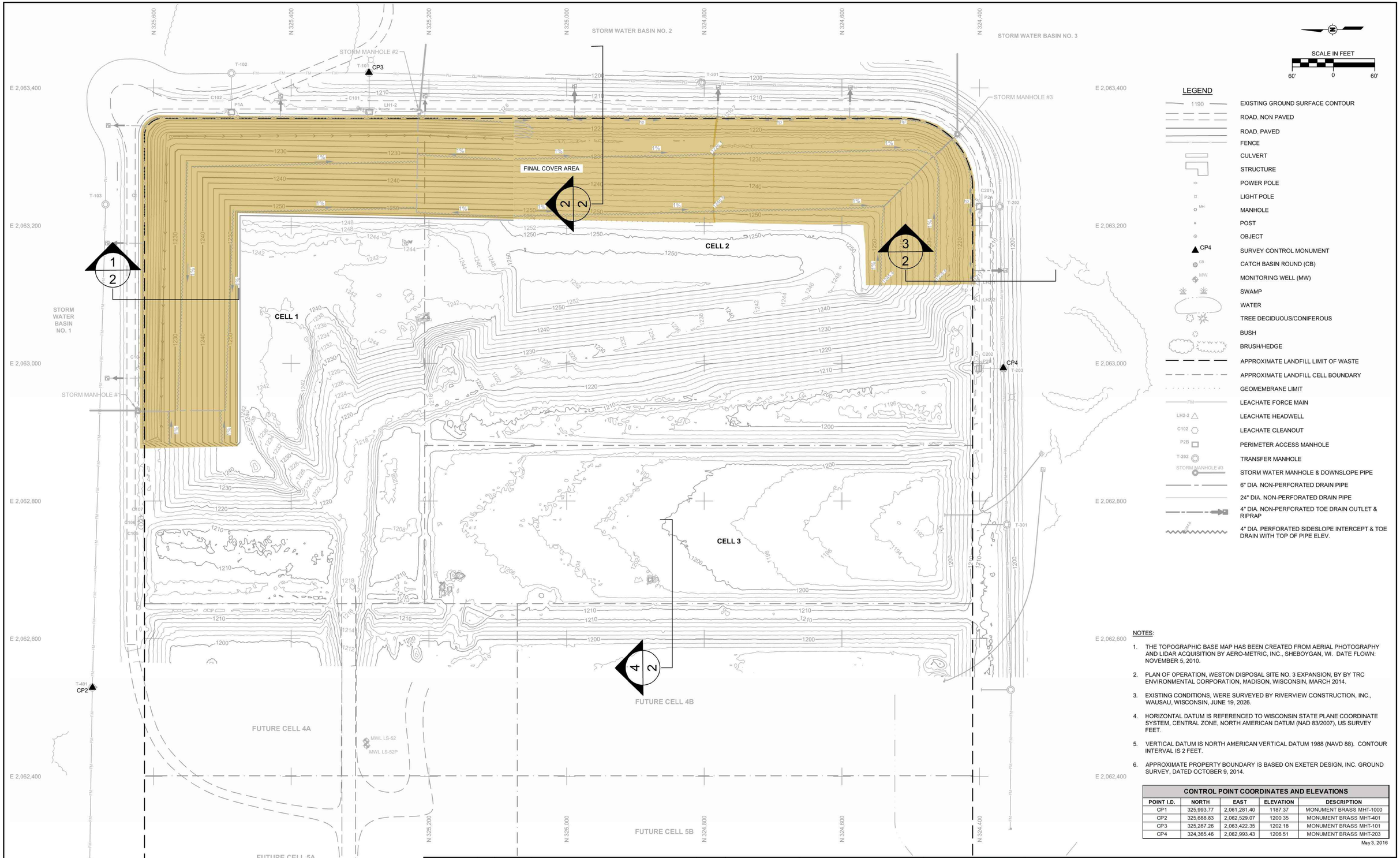
The analyses presented in this report indicate that the run-on controls prevent flow onto the active landfill during the peak discharge from the 24-hour, 25-year storm, and the run-off controls collect and control at least the volume generated by that storm. These results support compliance with § 257.81. Collected leachate is either reused for dust control within the active landfill or hauled to the Weston Power Plant wastewater treatment facility in accordance with the approved operating plan and § 257.3-3.

6. References

Perica, S., D. Martin, S. Pavlovic, I. Roy, M. St. Laurent, C. Trypaluk, D. Unruh, M. Yekta, G. Bonnin (2013). NOAA Atlas 14 Volume 8 Version 2.0, *Precipitation-Frequency Atlas of the United States, Midwestern States*. National Oceanic and Atmospheric Administration, National Weather Service, Silver Spring, Maryland.

US Department of Commerce. National Oceanic and Atmospheric Administration, National Weather Service. (2016). Precipitation Frequency Data Server (PFDS). http://hdsc.nws.noaa.gov/hdsc/pdfs/.

Appendix A Drawing C-1 – Weston Disposal Site No. 3 Cells 1 – 3



NOTES:

1. THE TOPOGRAPHIC BASE MAP HAS BEEN CREATED FROM AERIAL PHOTOGRAPHY AND LIDAR ACQUISITION BY AERO-METRIC, INC., SHEBOYGAN, WI. DATE FLOWN: NOVEMBER 5, 2010.
2. PLAN OF OPERATION, WESTON DISPOSAL SITE NO. 3 EXPANSION, BY BY TRC ENVIRONMENTAL CORPORATION, MADISON, WISCONSIN, MARCH 2014.
3. EXISTING CONDITIONS, WERE SURVEYED BY RIVERVIEW CONSTRUCTION, INC., WAUSAU, WISCONSIN, JUNE 19, 2026.
4. HORIZONTAL DATUM IS REFERENCED TO WISCONSIN STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE, NORTH AMERICAN DATUM (NAD 83/2007), US SURVEY FEET.
5. VERTICAL DATUM IS NORTH AMERICAN VERTICAL DATUM 1988 (NAVD 88). CONTOUR INTERVAL IS 2 FEET.
6. APPROXIMATE PROPERTY BOUNDARY IS BASED ON EXETER DESIGN, INC. GROUND SURVEY, DATED OCTOBER 9, 2014.

CONTROL POINT COORDINATES AND ELEVATIONS

POINT I.D.	NORTH	EAST	ELEVATION	DESCRIPTION
CP1	325,993.77	2,061,281.40	1187.37	MONUMENT BRASS MHT-1000
CP2	325,688.83	2,062,529.07	1200.35	MONUMENT BRASS MHT-401
CP3	325,287.26	2,063,422.35	1202.18	MONUMENT BRASS MHT-101
CP4	324,365.46	2,062,993.43	1206.51	MONUMENT BRASS MHT-203

May 3, 2016

Attention:

0 1"

If this scale bar does not measure 1" then drawing is not original scale.

NO.	DATE	ISSUE/REVISION	APP
0	X	X	X

Designed: JXT
Checked: JXT
Drawn: JLC
Submittal Date: 10/19/2026

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WE ENERGIES
333 WEST EVERETT STREET
MILWAUKEE, WISCONSIN

GEI Project 1600630

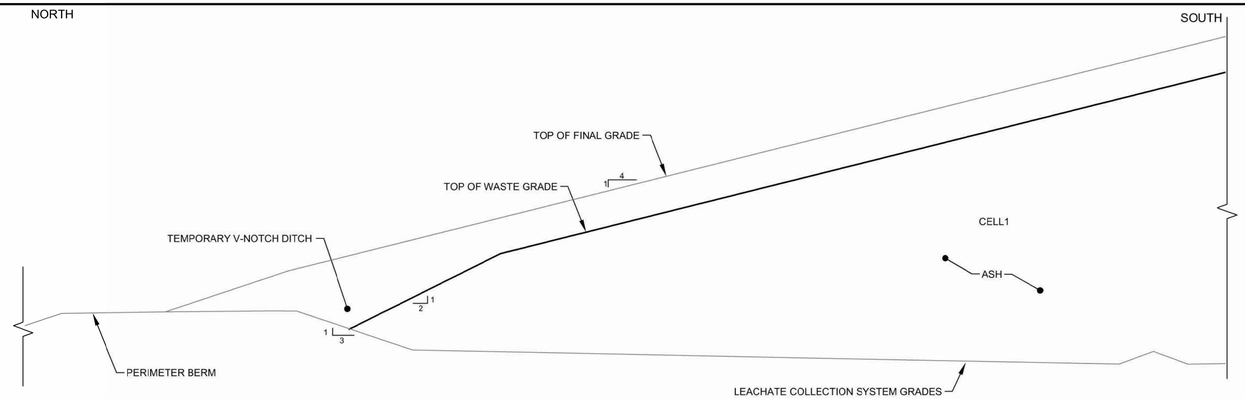
REGULATION COMPLIANCE REPORT
RUN-ON AND RUN-OFF CONTROL PLAN

DWG. NO. C-1

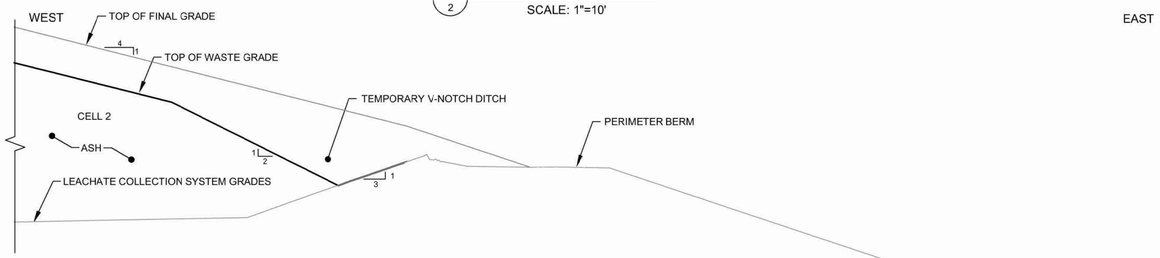
WESTON DISPOSAL SITE NO. 3
CELLS 1 - 3

SHEET NO. 1

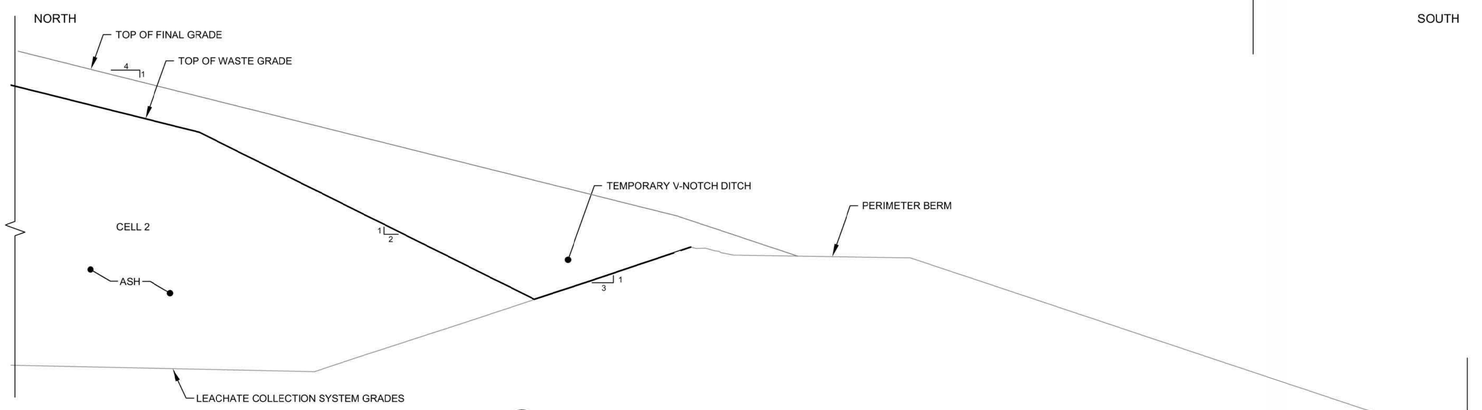
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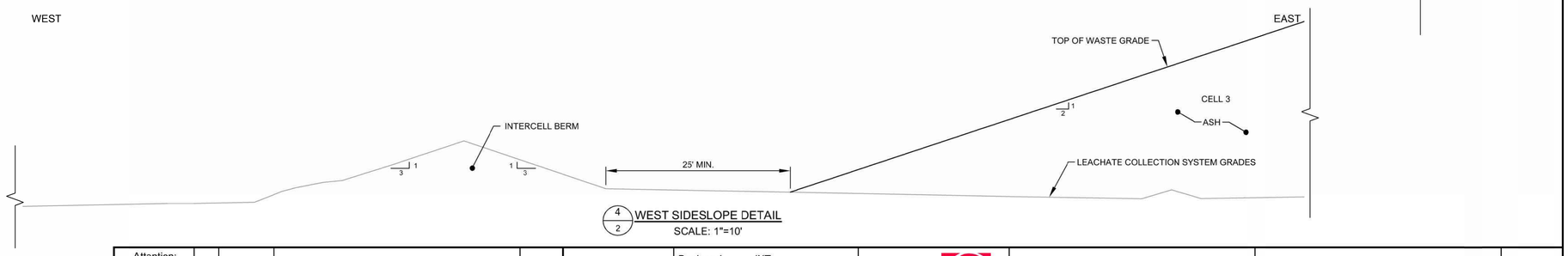
1/2 NORTH SIDESLOPE DETAIL
SCALE: 1"=10'



2/2 EAST SIDESLOPE DETAIL
SCALE: 1"=10'



3/2 SOUTH SIDESLOPE DETAIL
SCALE: 1"=10'



4/2 WEST SIDESLOPE DETAIL
SCALE: 1"=10'

Attention:

0 1"

If this scale bar does not measure 1" then drawing is not original scale.

NO.	DATE	ISSUE/REVISION	APP
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Designed:	JXT
Checked:	JXT
Drawn:	JLC
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GEI Project 1600630

REGULATION COMPLIANCE REPORT
RUN-ON AND RUN-OFF CONTROL PLAN

DWG. NO. C-2

WESTON DISPOSAL SITE NO. 3
CELLS 1 - 3 DETAILS

SHEET NO. 2

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Appendix B NOAA 14, Vol 8 Rainfall Analysis and Run-off Volume

GEI Consultants	**Client**	WEC Energy Group			**Page**	1 of 4
	Project	Weston Disposal Site No. 3 Run-on and Run-off Control Plan			**Pg. Rev.**	0
	By	A. Schwoerer	**Chk.**	J. Trast	**App.**	J. Trast
	Date	10/15/2026	**Date**	10/26/26	**Date**	10/26/26
GEI Project No.	2103691	**Document No.**	N/A			
Subject	NOAA 14, Vol. 8 Rainfall Analysis and Run-off Volume					

Purpose:

The purpose of this calculation is to estimate the 24-hour, 25-year precipitation event at Weston Disposal Site No. 3 (WDS3). The 24-hour, 25-year precipitation event is required for the run-on and run-off control system plan for the landfill.

Procedure:

The rainfall depth estimation follows the procedures outlined in Precipitation-Frequency (PF) Atlas of the United States (Atlas 14, Volume 8, Version 2: Wisconsin).

As instructed in Atlas 14, the user is referred to the NOAA Precipitation Frequency Data Server (PFDS) http://hdsc.nws.noaa.gov/hdsc/pfds/index.html. The approximate center of the landfill was input into the PFDS and the PF estimates were returned.

Landfill Centroid Coordinates

44°43'27.12"N	44.7242°
89°38'12.84"W	-89.6369°



Tabular Output from the PFDS:

PDS-based precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**0.315** (0.260-0.386)	**0.372** (0.306-0.456)	**0.470** (0.386-0.577)	**0.557** (0.454-0.685)	**0.684** (0.542-0.869)	**0.788** (0.608-1.01)	**0.897** (0.669-1.17)	**1.01** (0.724-1.34)	**1.18** (0.807-1.58)	**1.31** (0.869-1.77)
10-min	**0.462** (0.381-0.566)	**0.545** (0.448-0.668)	**0.688** (0.565-0.845)	**0.815** (0.665-1.00)	**1.00** (0.793-1.27)	**1.15** (0.891-1.48)	**1.31** (0.979-1.71)	**1.48** (1.06-1.96)	**1.72** (1.18-2.32)	**1.91** (1.27-2.59)
15-min	**0.563** (0.464-0.690)	**0.664** (0.547-0.814)	**0.839** (0.689-1.03)	**0.994** (0.811-1.22)	**1.22** (0.968-1.55)	**1.41** (1.09-1.80)	**1.60** (1.19-2.08)	**1.81** (1.29-2.39)	**2.10** (1.44-2.83)	**2.33** (1.55-3.16)
30-min	**0.793** (0.654-0.971)	**0.934** (0.769-1.14)	**1.18** (0.968-1.45)	**1.40** (1.14-1.72)	**1.72** (1.36-2.18)	**1.98** (1.53-2.53)	**2.26** (1.68-2.93)	**2.55** (1.82-3.38)	**2.96** (2.03-3.99)	**3.29** (2.19-4.46)
60-min	**1.02** (0.840-1.25)	**1.19** (0.981-1.46)	**1.50** (1.23-1.84)	**1.77** (1.44-2.18)	**2.18** (1.73-2.78)	**2.52** (1.95-3.23)	**2.88** (2.15-3.75)	**3.27** (2.34-4.33)	**3.82** (2.62-5.15)	**4.26** (2.83-5.76)
2-hr	**1.25** (1.03-1.51)	**1.45** (1.20-1.76)	**1.81** (1.50-2.20)	**2.14** (1.76-2.61)	**2.64** (2.12-3.34)	**3.05** (2.38-3.88)	**3.50** (2.64-4.52)	**3.98** (2.88-5.24)	**4.67** (3.24-6.25)	**5.22** (3.52-7.01)
3-hr	**1.38** (1.15-1.67)	**1.60** (1.33-1.93)	**1.99** (1.65-2.40)	**2.34** (1.94-2.84)	**2.89** (2.33-3.64)	**3.35** (2.63-4.24)	**3.85** (2.92-4.95)	**4.38** (3.19-5.74)	**5.16** (3.60-6.87)	**5.78** (3.91-7.72)
6-hr	**1.64** (1.38-1.96)	**1.89** (1.58-2.26)	**2.33** (1.95-2.79)	**2.74** (2.28-3.29)	**3.36** (2.73-4.20)	**3.89** (3.08-4.88)	**4.46** (3.41-5.69)	**5.08** (3.73-6.60)	**5.97** (4.22-7.89)	**6.69** (4.58-8.86)
12-hr	**1.94** (1.64-2.30)	**2.23** (1.88-2.64)	**2.74** (2.31-3.25)	**3.21** (2.68-3.82)	**3.90** (3.20-4.82)	**4.49** (3.58-5.57)	**5.12** (3.95-6.46)	**5.80** (4.30-7.45)	**6.76** (4.82-8.84)	**7.54** (5.22-9.90)
24-hr	**2.28** (1.94-2.67)	**2.60** (2.22-3.06)	**3.18** (2.70-3.74)	**3.70** (3.12-4.36)	**4.47** (3.68-5.45)	**5.11** (4.10-6.26)	**5.78** (4.50-7.21)	**6.50** (4.86-8.26)	**7.52** (5.42-9.74)	**8.33** (5.84-10.8)
2-day	**2.64** (2.27-3.07)	**3.01** (2.58-3.50)	**3.65** (3.12-4.25)	**4.22** (3.59-4.93)	**5.06** (4.20-6.10)	**5.76** (4.67-6.99)	**6.49** (5.10-8.01)	**7.27** (5.49-9.15)	**8.36** (6.09-10.7)	**9.24** (6.55-11.9)
3-day	**2.89** (2.49-3.34)	**3.30** (2.84-3.81)	**4.00** (3.44-4.63)	**4.62** (3.95-5.37)	**5.54** (4.62-6.63)	**6.29** (5.12-7.59)	**7.08** (5.58-8.68)	**7.91** (6.01-9.90)	**9.08** (6.66-11.6)	**10.0** (7.15-12.9)
4-day	**3.12** (2.69-3.58)	**3.55** (3.07-4.09)	**4.31** (3.71-4.96)	**4.97** (4.26-5.75)	**5.94** (4.96-7.07)	**6.72** (5.50-8.08)	**7.55** (5.98-9.23)	**8.43** (6.43-10.5)	**9.64** (7.10-12.2)	**10.6** (7.61-13.6)
7-day	**3.76** (3.27-4.29)	**4.25** (3.70-4.86)	**5.09** (4.42-5.83)	**5.82** (5.02-6.68)	**6.86** (5.77-8.09)	**7.70** (6.33-9.15)	**8.56** (6.83-10.4)	**9.47** (7.28-11.7)	**10.7** (7.96-13.5)	**11.7** (8.47-14.9)
10-day	**4.36** (3.81-4.95)	**4.89** (4.27-5.56)	**5.78** (5.03-6.58)	**6.54** (5.66-7.46)	**7.62** (6.42-8.91)	**8.47** (6.99-10.0)	**9.34** (7.49-11.2)	**10.3** (7.92-12.6)	**11.5** (8.58-14.4)	**12.5** (9.07-15.7)
20-day	**6.08** (5.35-6.83)	**6.71** (5.90-7.54)	**7.73** (6.79-8.71)	**8.58** (7.50-9.70)	**9.75** (8.27-11.2)	**10.6** (8.86-12.4)	**11.5** (9.33-13.7)	**12.5** (9.71-15.1)	**13.6** (10.3-16.9)	**14.6** (10.7-18.2)
30-day	**7.49** (6.63-8.37)	**8.23** (7.28-9.20)	**9.42** (8.31-10.6)	**10.4** (9.11-11.7)	**11.7** (9.94-13.4)	**12.7** (10.6-14.6)	**13.6** (11.0-16.0)	**14.5** (11.4-17.5)	**15.7** (11.9-19.3)	**16.6** (12.3-20.7)
45-day	**9.25** (8.22-10.3)	**10.2** (9.04-11.3)	**11.6** (10.3-13.0)	**12.8** (11.3-14.3)	**14.3** (12.2-16.3)	**15.5** (13.0-17.7)	**16.5** (13.5-19.3)	**17.5** (13.8-20.9)	**18.8** (14.3-22.9)	**19.7** (14.7-24.4)
60-day	**10.7** (9.57-11.9)	**11.9** (10.6-13.1)	**13.6** (12.1-15.1)	**15.0** (13.3-16.7)	**16.8** (14.3-18.9)	**18.0** (15.2-20.6)	**19.2** (15.7-22.3)	**20.3** (16.1-24.1)	**21.7** (16.6-26.3)	**22.6** (17.0-27.9)

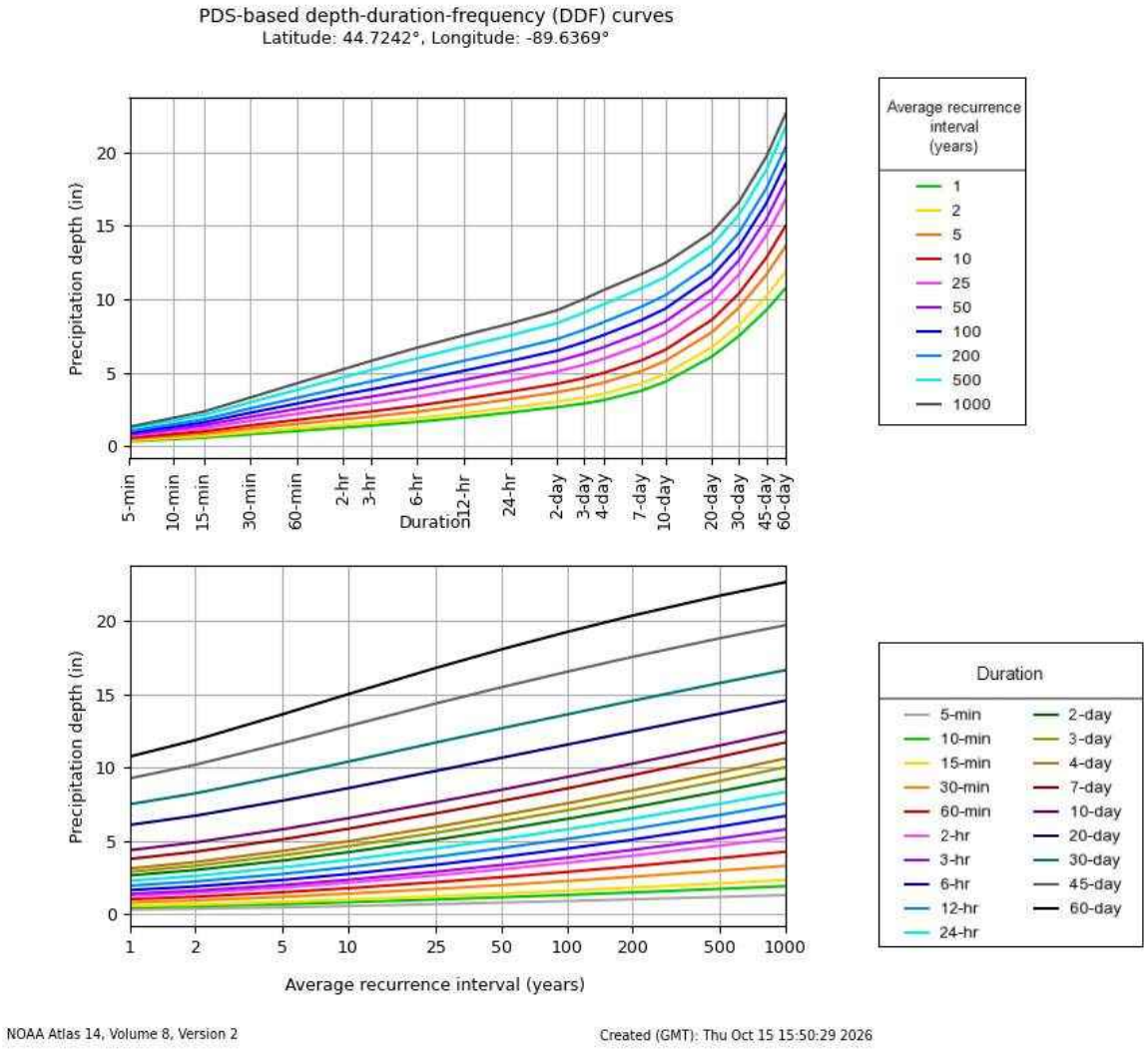
[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

GEI Consultants	**Client**	WEC Energy Group			**Page**	3 of 4
	Project	Weston Disposal Site No. 3 Run-on and Run-off Control Plan			**Pg. Rev.**	0
	By	A. Schwoerer	**Chk.**	J. Trast	**App.**	J. Trast
	Date	10/15/2026	**Date**	10/26/26	**Date**	10/26/26
GEI Project No.	2103691	**Document No.**	N/A			
Subject	NOAA 14, Vol. 8 Rainfall Analysis and Run-off Volume					

Graphical Output from the PFDS:



GEI Consultants	Client	WEC Energy Group			Page	4 of 4
	Project	Weston Disposal Site No. 3 Run-on and Run-off Control Plan			Pg. Rev.	0
	By	A. Schwoerer	Chk.	J. Trast	App.	J. Trast
	Date	10/15/2026	Date	10/26/26	Date	10/26/26
GEI Project No.	2103691	Document No.	N/A			
Subject	NOAA 14, Vol. 8 Rainfall Analysis and Run-off Volume					

Regulations:

The Weston Disposal Site No. 3 is regulated under 40 CFR Part 257 Subpart D – Standards for Disposal of Coal Combustion Residuals (CCR) in Landfills and Surface Impoundments as an existing landfill. The regulations specify that landfill must have the following plans in place:

- A run-on control system to prevent flow onto the active portion of the CCR unit during the peak discharge from a 24-hour, 25-year storm.
- A run-off control system from the active portion of the CCR unit to collect and control at least the water volume resulting from a 24-hour, 25-year storm.

Conclusion:

The 24-hour, 25-year storm for the Weston Disposal Site No. 3 is 4.47 inches. This value will be utilized in the stormwater run-off model (under a separate calculation package).

Appendix C Stormwater Run-on Calculations

GEI Consultants	Client	WEC Energy Group			Page	1 of 1
	Project	WDS3 LF Run-on and Run-off Control Plan			Rev.	0
	By	A. Schwoerer	Chk.	J. Trast	App.	J. Trast
	Date	10/23/2026	Date	10/26/2026	Date	10/26/2026
GEI Project No.	2103691	Document No.	N/A			
Subject	Stormwater Run-on Calculations					

Purpose:

The purpose of this calculation is to model and confirm the current run-on control system for the Weston Disposal Site No. 3 Ash Landfill can adequately manage a 24-hr, 25-yr precipitation event.

Design Criteria and Assumptions:

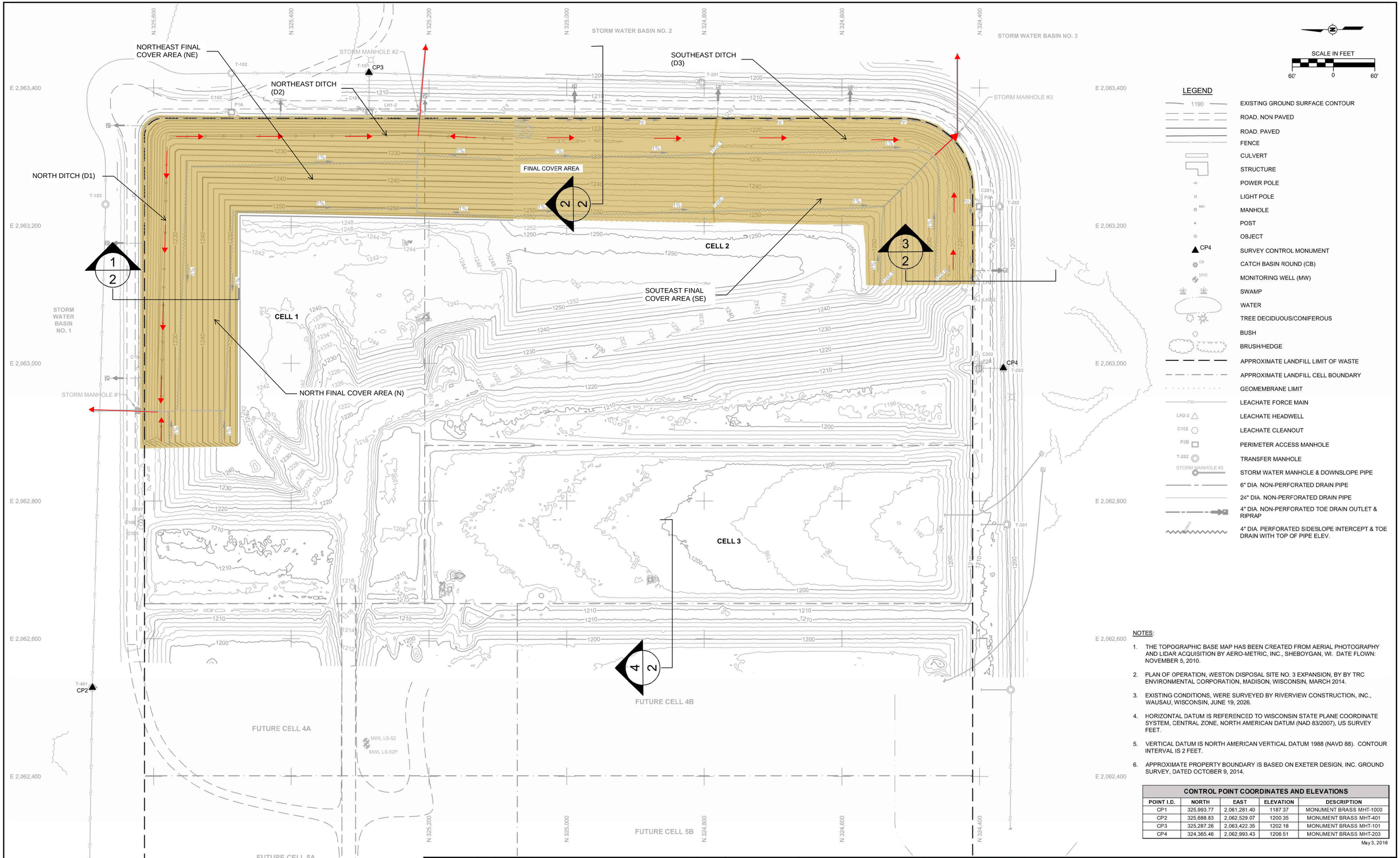
1. The rainfall depth estimation for the 24-hr, 25-yr event is 4.47 inches. The rainfall depth was determined by following procedures outlined in Precipitation-Frequency (PF) Atlas of the United States (Atlas 14, Volume 8, Version 2: Wisconsin). (See Appendix B)
2. Permanent perimeter berms have been established around the north, east, and south sides of the landfill to direct stormwater run-on away from the landfill. Temporary intercell berms perform the same function on the west side of Cell 1 and Cell 3 and prevent run-on from entering the active landfill.
3. Approximately 5.6 acres of the Cells 1 and 2 perimeter slopes have received final cover. The stormwater flow from the final cover is routed to a diversion berm ditch located near the toe of the cover and discharges into either Stormwater Basin No. 1, No. 2, or No. 3.
4. Stormwater modeling was completed to confirm that the current run-on control system is sufficiently sized to manage a 24-hr, 25-yr precipitation event.
5. HydroCAD 10.0 was used to model the stormwater run-on.
6. The stormwater run-on control ditch is 3-feet-deep with a 4H:1V slope and the final cover side, and a 2H:1V slope on the diversion berm side.
7. Subcatchment and reach parameters are included in the attached HydroCAD Report.

Results:

Based on stormwater model, the current run-on control system at the Weston Disposal Site No.3 Ash Landfill is able to handle the 24-hr, 25-yr precipitation event without allowing run-on stormwater to enter the landfill. The estimated peak water level in the North Ditch, Northeast Ditch, and Southeast Ditch are 0.97 feet, 1.06 feet, and 1.35 feet, respectively; which is less than the 3-feet-deep channel. The diversion berm ditches are adequately sized to prevent run-on into the landfill associated with the 24-hour, 25-year precipitation event.

Attachments:

- Figure 1 – Stormwater Flow Diagram
- HydroCAD Summary Report



NOTES:

1. THE TOPOGRAPHIC BASE MAP HAS BEEN CREATED FROM AERIAL PHOTOGRAPHY AND LIDAR ACQUISITION BY AERO-METRIC, INC., SHEBOYGAN, WI. DATE FLOWN: NOVEMBER 5, 2010.
2. PLAN OF OPERATION, WESTON DISPOSAL SITE NO. 3 EXPANSION, BY BY TRC ENVIRONMENTAL CORPORATION, MADISON, WISCONSIN, MARCH 2014.
3. EXISTING CONDITIONS, WERE SURVEYED BY RIVERVIEW CONSTRUCTION, INC., WAUSAU, WISCONSIN, JUNE 19, 2026.
4. HORIZONTAL DATUM IS REFERENCED TO WISCONSIN STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE, NORTH AMERICAN DATUM (NAD 83/2007), US SURVEY FEET.
5. VERTICAL DATUM IS NORTH AMERICAN VERTICAL DATUM 1988 (NAVD 88). CONTOUR INTERVAL IS 2 FEET.
6. APPROXIMATE PROPERTY BOUNDARY IS BASED ON EXETER DESIGN, INC. GROUND SURVEY, DATED OCTOBER 9, 2014.

CONTROL POINT COORDINATES AND ELEVATIONS

POINT I.D.	NORTH	EAST	ELEVATION	DESCRIPTION
CP1	325,993.77	2,061,281.40	1187.37	MONUMENT BRASS MHT-1000
CP2	325,688.83	2,062,529.07	1200.35	MONUMENT BRASS MHT-401
CP3	325,287.26	2,063,422.35	1202.18	MONUMENT BRASS MHT-101
CP4	324,365.46	2,062,993.43	1206.51	MONUMENT BRASS MHT-203

May 3, 2016

Attention:	NO.	DATE	ISSUE/REVISION	APP
0 1"	0	X	X	X
If this scale bar does not measure 1" then drawing is not original scale.				

Designed: JXT
Checked: JXT
Drawn: JLC
Submittal Date: 10/19/2026

GEI Consultants
3159 Voyager Drive
Green Bay, Wisconsin 54311
920-455-8200

WE ENERGIES
333 WEST EVERETT STREET
MILWAUKEE, WISCONSIN

GEI Project 1600630

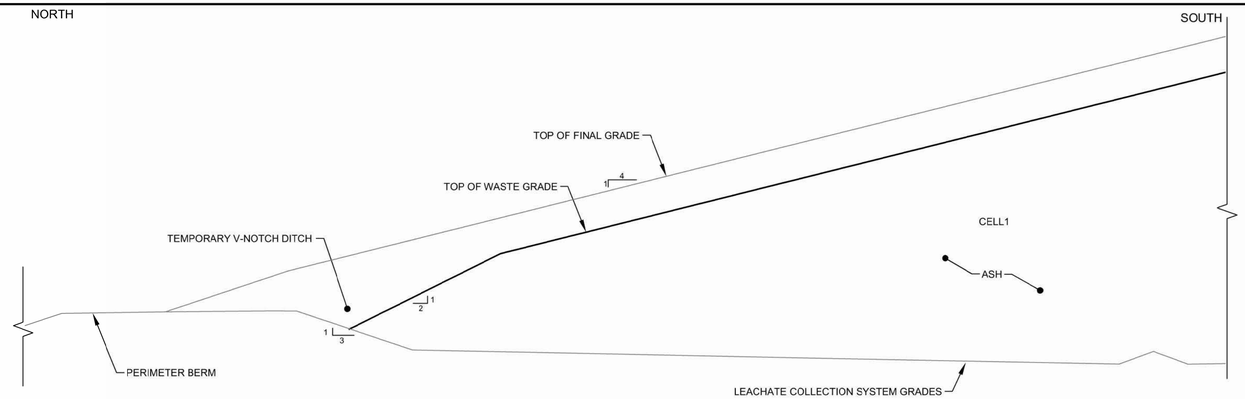
REGULATION COMPLIANCE REPORT
RUN-ON AND RUN-OFF CONTROL PLAN

DWG. NO. C-1

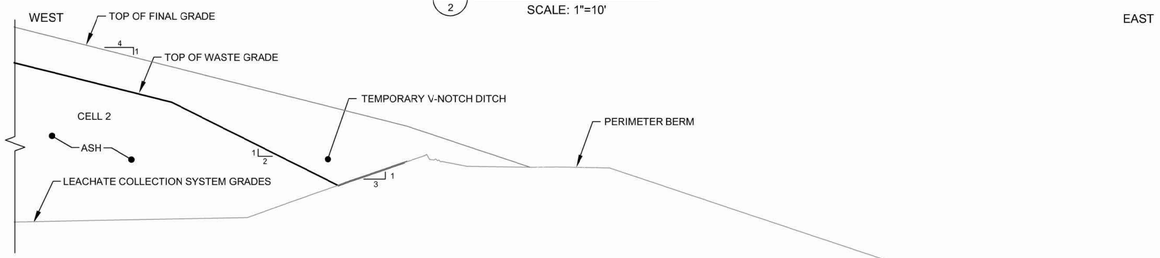
WESTON DISPOSAL SITE NO. 3
CELLS 1 - 3

SHEET NO. 1

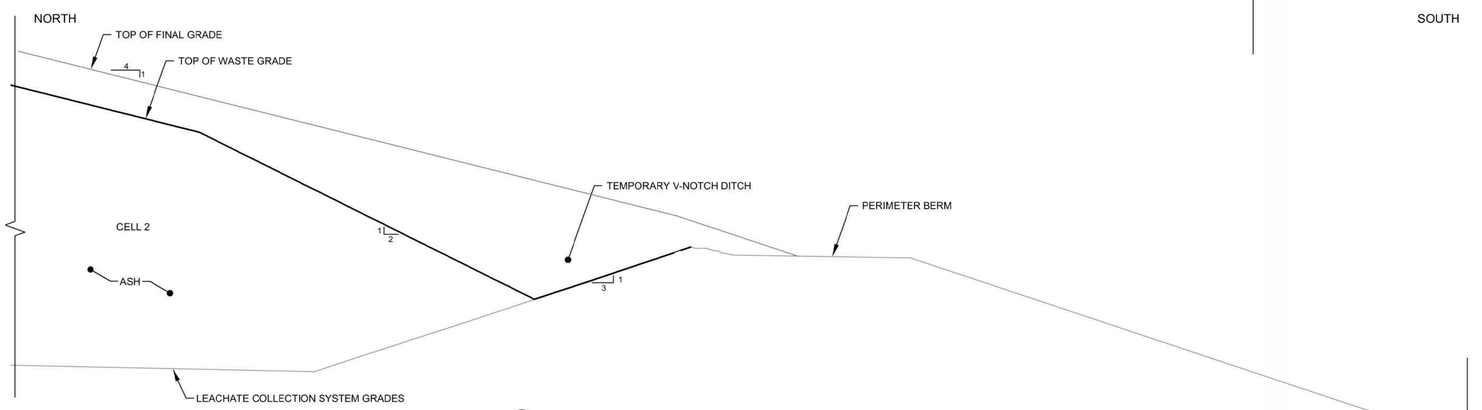
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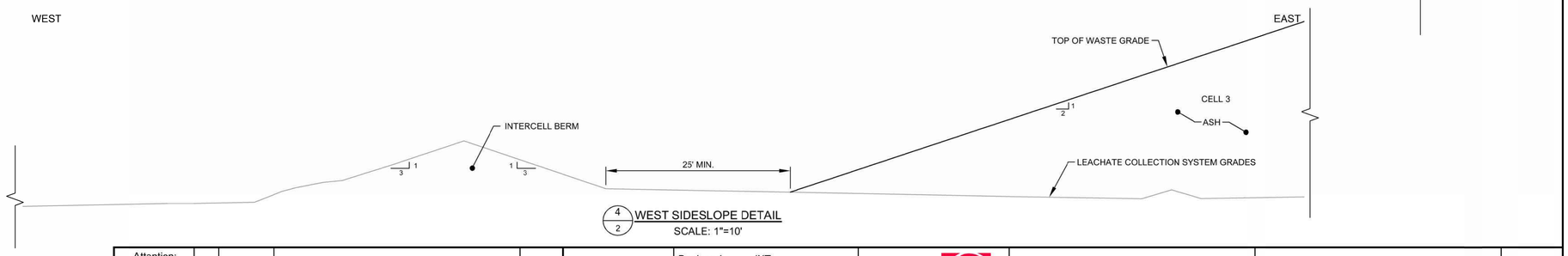
1/2 NORTH SIDESLOPE DETAIL
SCALE: 1"=10'



2/2 EAST SIDESLOPE DETAIL
SCALE: 1"=10'



3/2 SOUTH SIDESLOPE DETAIL
SCALE: 1"=10'



4/2 WEST SIDESLOPE DETAIL
SCALE: 1"=10'

Attention:

0 1"

If this scale bar does not measure 1" then drawing is not original scale.

NO.	DATE	ISSUE/REVISION	APP
0	X	X	X

Designed:	JXT
Checked:	JXT
Drawn:	JLC
Submittal Date:	10/19/2026

GEI Consultants
3159 Voyager Drive
Green Bay, Wisconsin 54311
920-455-8200

WE ENERGIES
333 WEST EVERETT STREET
MILWAUKEE, WISCONSIN

GEI Project 1600630

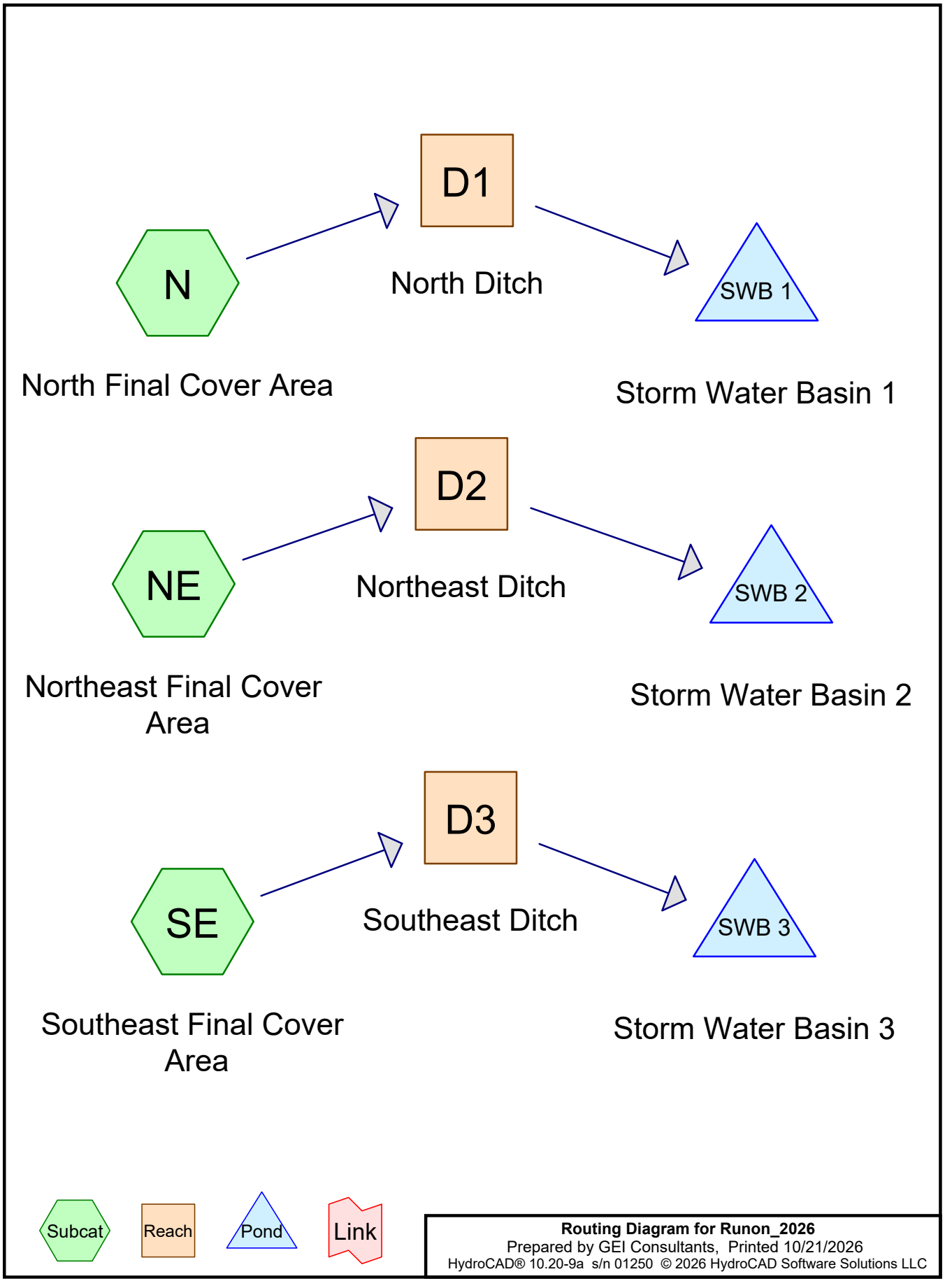
REGULATION COMPLIANCE REPORT
RUN-ON AND RUN-OFF CONTROL PLAN

DWG. NO. C-2

WESTON DISPOSAL SITE NO. 3
CELLS 1 - 3 DETAILS

SHEET NO. 2

Filename: B:\WORKING\WEC ENERGY GROUP_GENERAL\WE - WESTON DISPOSAL SITE NO 3\1600630_RUN-ON RUN-OFF CONTROL PLAN\DWG\2026\1600630002_2026.DWG Last saved by: JAMCAL2032 Last Plotted: 2026-10-12


D1
N
North Ditch
SWB 1
North Final Cover Area
Storm Water Basin 1
D2
NE
Northeast Ditch
SWB 2
Northeast Final Cover Area
Storm Water Basin 2
D3
SE
Southeast Ditch
SWB 3
Southeast Final Cover Area
Storm Water Basin 3
Subcat
Reach
Pond
Link
Routing Diagram for Runon_2026
Prepared by GEI Consultants, Printed 10/21/2026
HydroCAD® 10.20-9a s/n 01250 © 2026 HydroCAD Software Solutions LLC

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
5.571	91	Newly graded area, HSG C (N, NE, SE)
5.571	**91**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
5.571	HSG C	N, NE, SE
0.000	HSG D	
0.000	Other	
5.571		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	5.571	0.000	0.000	5.571	Newly graded area	N, NE, SE
0.000	**0.000**	**5.571**	**0.000**	**0.000**	**5.571**	**TOTAL AREA**	

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentN: North Final Cover Area Runoff Area=58,421 sf 0.00% Impervious Runoff Depth=3.47"
Flow Length=440' Slope=0.0045 '/' Tc=0.6 min CN=91 Runoff=8.71 cfs 0.388 af

SubcatchmentNE: Northeast Final Cover Runoff Area=68,332 sf 0.00% Impervious Runoff Depth=3.47"
Flow Length=500' Slope=0.0040 '/' Tc=0.7 min CN=91 Runoff=10.16 cfs 0.453 af

SubcatchmentSE: Southeast Final Cover Runoff Area=115,913 sf 0.00% Impervious Runoff Depth=3.47"
Flow Length=800' Slope=0.0025 '/' Tc=1.4 min CN=91 Runoff=16.74 cfs 0.769 af

Reach D1: North Ditch Avg. Flow Depth=0.97' Max Vel=2.69 fps Inflow=8.71 cfs 0.388 af
n=0.022 L=440.0' S=0.0045 '/' Capacity=154.99 cfs Outflow=7.58 cfs 0.388 af

Reach D2: Northeast Ditch Avg. Flow Depth=1.06' Max Vel=2.66 fps Inflow=10.16 cfs 0.453 af
n=0.022 L=500.0' S=0.0040 '/' Capacity=145.40 cfs Outflow=8.89 cfs 0.453 af

Reach D3: Southeast Ditch Avg. Flow Depth=1.35' Max Vel=2.50 fps Inflow=16.74 cfs 0.769 af
n=0.022 L=800.0' S=0.0025 '/' Capacity=114.95 cfs Outflow=13.31 cfs 0.769 af

Pond SWB 1: Storm Water Basin 1 Inflow=7.58 cfs 0.388 af
Primary=7.58 cfs 0.388 af

Pond SWB 2: Storm Water Basin 2 Inflow=8.89 cfs 0.453 af
Primary=8.89 cfs 0.453 af

Pond SWB 3: Storm Water Basin 3 Inflow=13.31 cfs 0.769 af
Primary=13.31 cfs 0.769 af

Total Runoff Area = 5.571 ac Runoff Volume = 1.610 af Average Runoff Depth = 3.47"
100.00% Pervious = 5.571 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment N: North Final Cover Area

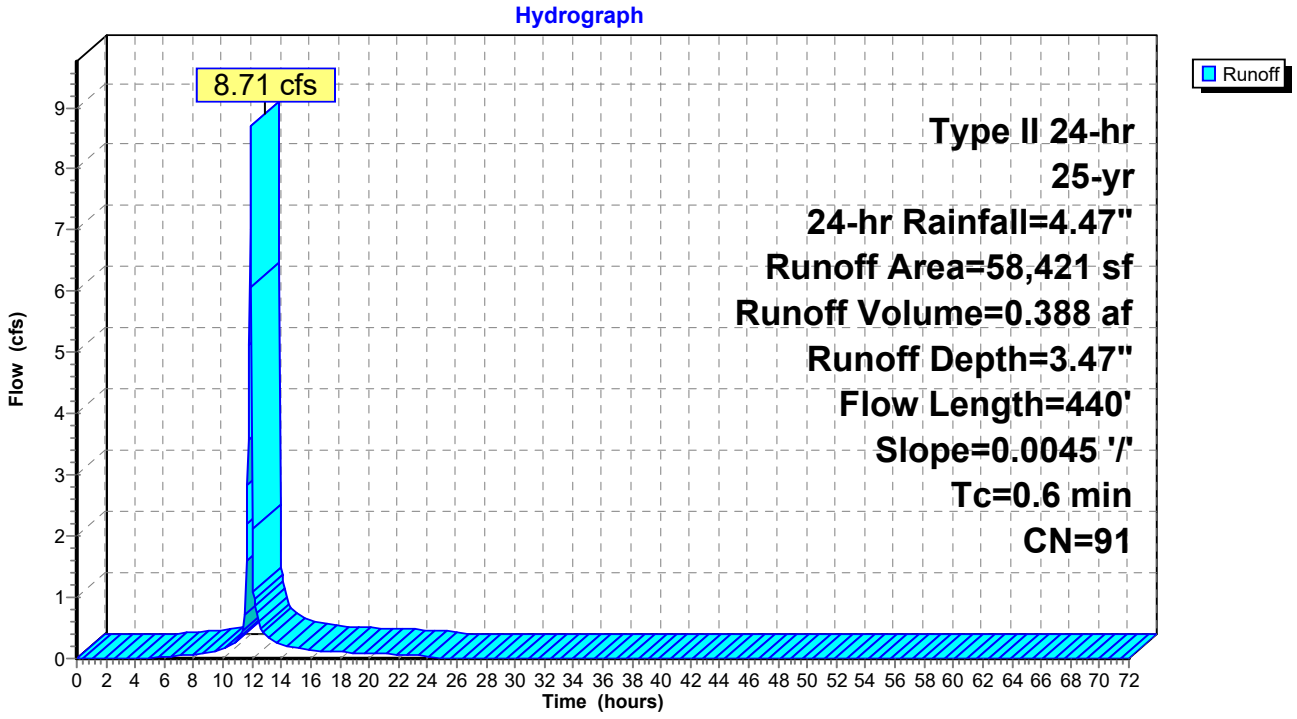
Runoff = 8.71 cfs @ 11.89 hrs, Volume= 0.388 af, Depth= 3.47"
Routed to Reach D1 : North Ditch

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (sf)	CN	Description
58,421	91	Newly graded area, HSG C
58,421		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.6	440	0.0045	12.35	333.46	**Channel Flow, North Ditch** Area= 27.0 sf Perim= 6.0' r= 4.50' n= 0.022 Earth, clean & straight

Subcatchment N: North Final Cover Area



Summary for Subcatchment NE: Northeast Final Cover Area

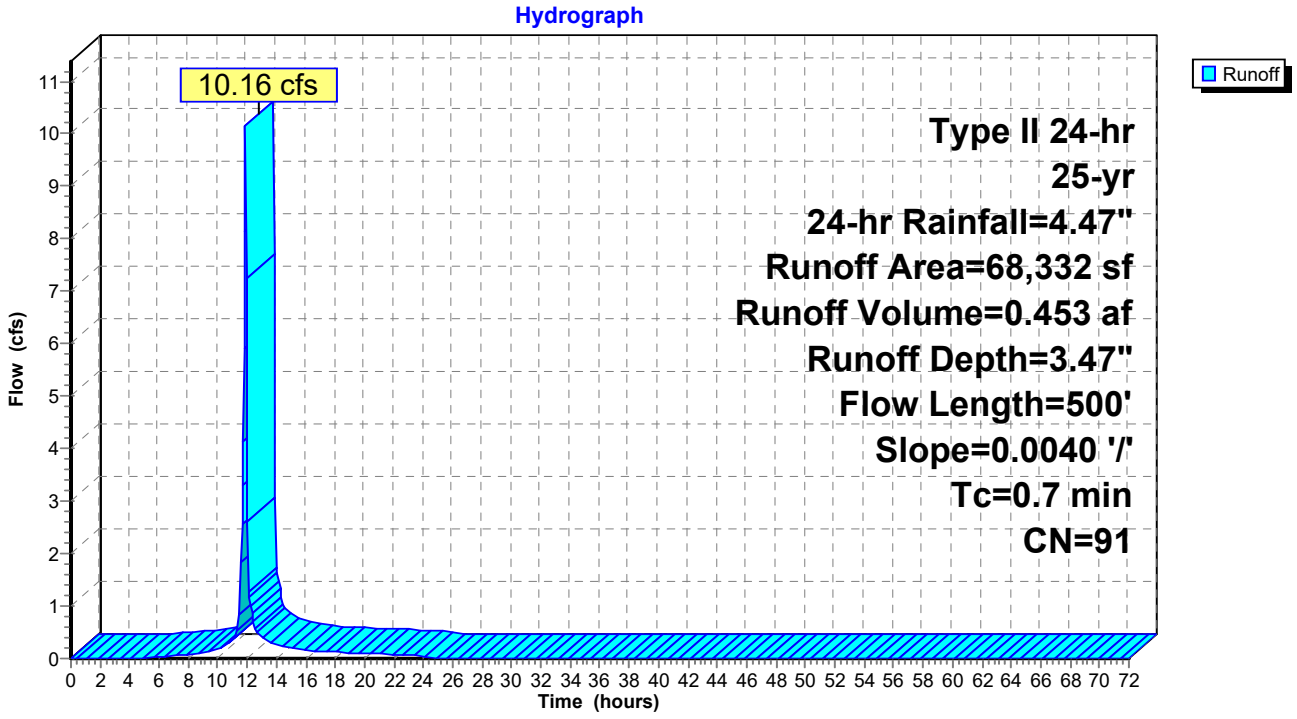
Runoff = 10.16 cfs @ 11.90 hrs, Volume= 0.453 af, Depth= 3.47"
Routed to Reach D2 : Northeast Ditch

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (sf)	CN	Description
68,332	91	Newly graded area, HSG C
68,332		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.7	500	0.0040	11.64	314.39	**Channel Flow, Northeast Ditch** Area= 27.0 sf Perim= 6.0' r= 4.50' n= 0.022 Earth, clean & straight

Subcatchment NE: Northeast Final Cover Area



Summary for Subcatchment SE: Southeast Final Cover Area

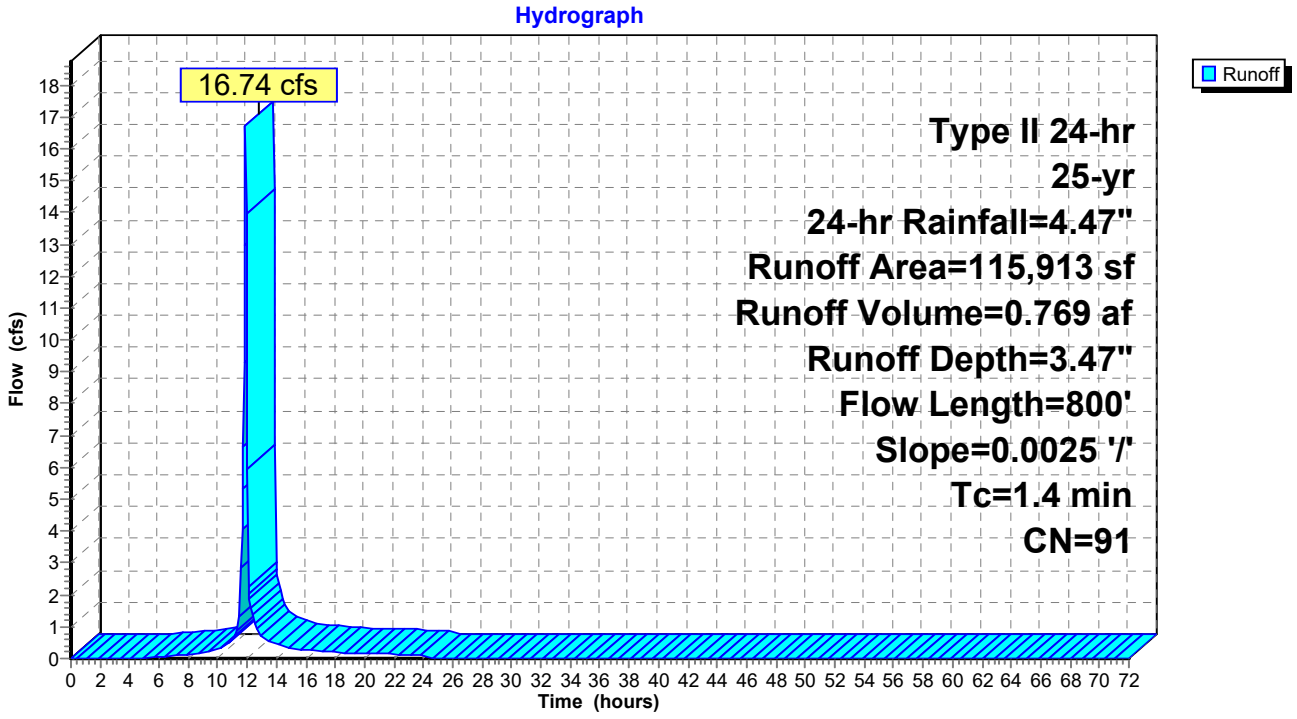
Runoff = 16.74 cfs @ 11.90 hrs, Volume= 0.769 af, Depth= 3.47"
Routed to Reach D3 : Southeast Ditch

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (sf)	CN	Description
115,913	91	Newly graded area, HSG C
115,913		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.4	800	0.0025	9.21	248.54	**Channel Flow, Southeast Ditch** Area= 27.0 sf Perim= 6.0' r= 4.50' n= 0.022 Earth, clean & straight

Subcatchment SE: Southeast Final Cover Area



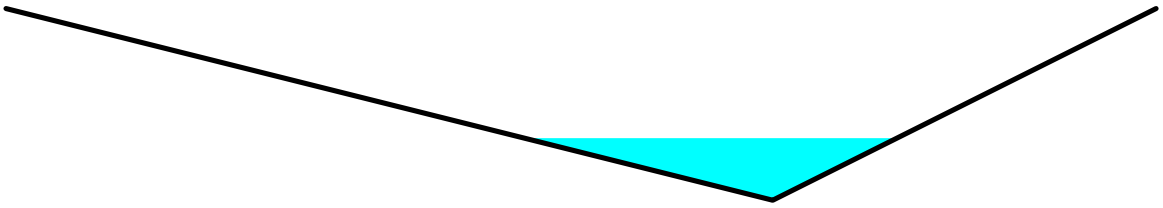
Summary for Reach D1: North Ditch

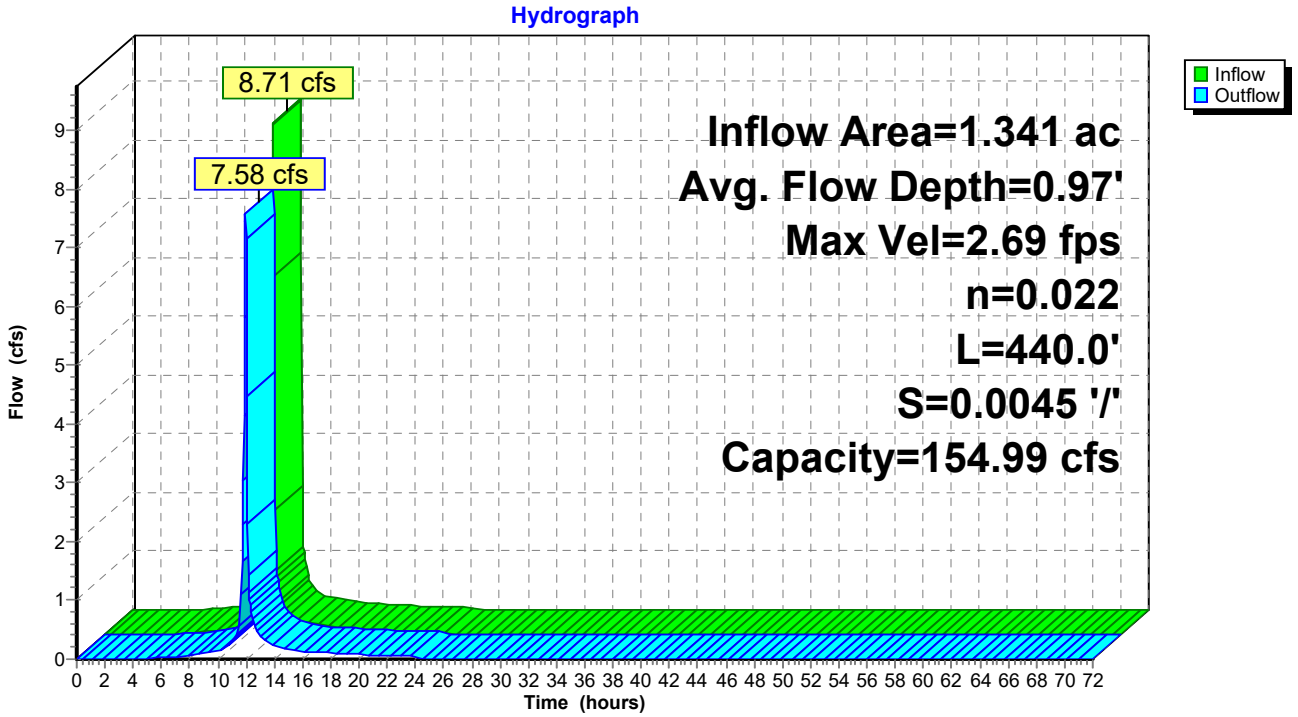
Inflow Area = 1.341 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 8.71 cfs @ 11.89 hrs, Volume= 0.388 af
Outflow = 7.58 cfs @ 11.97 hrs, Volume= 0.388 af, Atten= 13%, Lag= 4.3 min
Routed to Pond SWB 1 : Storm Water Basin 1

Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.69 fps, Min. Travel Time= 2.7 min
Avg. Velocity = 0.88 fps, Avg. Travel Time= 8.4 min

Peak Storage= 1,248 cf @ 11.92 hrs
Average Depth at Peak Storage= 0.97' , Surface Width= 5.84'
Bank-Full Depth= 3.00' Flow Area= 27.0 sf, Capacity= 154.99 cfs

0.00' x 3.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 4.0 2.0 '/' Top Width= 18.00'
Length= 440.0' Slope= 0.0045 '/'
Inlet Invert= 1,228.00', Outlet Invert= 1,226.00'





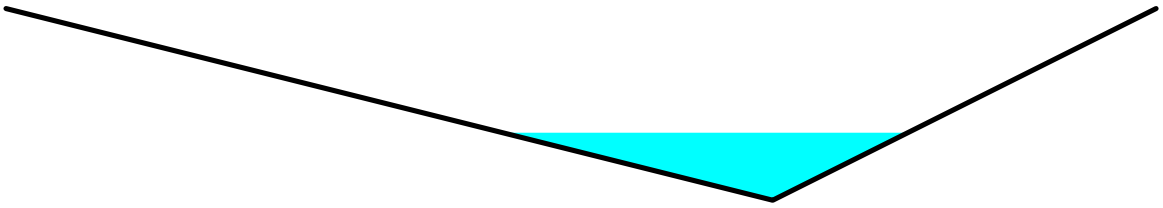
Summary for Reach D2: Northeast Ditch

Inflow Area = 1.569 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 10.16 cfs @ 11.90 hrs, Volume= 0.453 af
Outflow = 8.89 cfs @ 11.98 hrs, Volume= 0.453 af, Atten= 13%, Lag= 5.0 min
Routed to Pond SWB 2 : Storm Water Basin 2

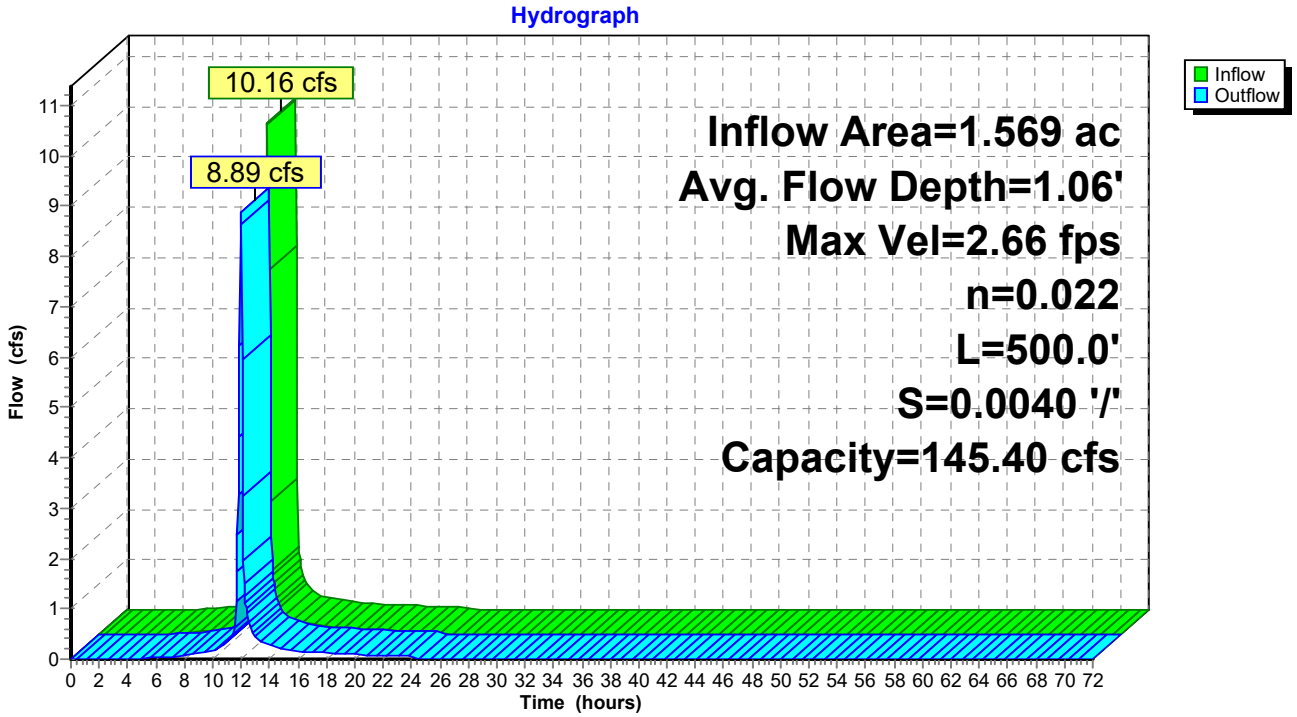
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.66 fps, Min. Travel Time= 3.1 min
Avg. Velocity = 0.86 fps, Avg. Travel Time= 9.7 min

Peak Storage= 1,669 cf @ 11.93 hrs
Average Depth at Peak Storage= 1.06' , Surface Width= 6.34'
Bank-Full Depth= 3.00' Flow Area= 27.0 sf, Capacity= 145.40 cfs

0.00' x 3.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 4.0 2.0 '/' Top Width= 18.00'
Length= 500.0' Slope= 0.0040 '/'
Inlet Invert= 1,226.00', Outlet Invert= 1,224.00'



Reach D2: Northeast Ditch



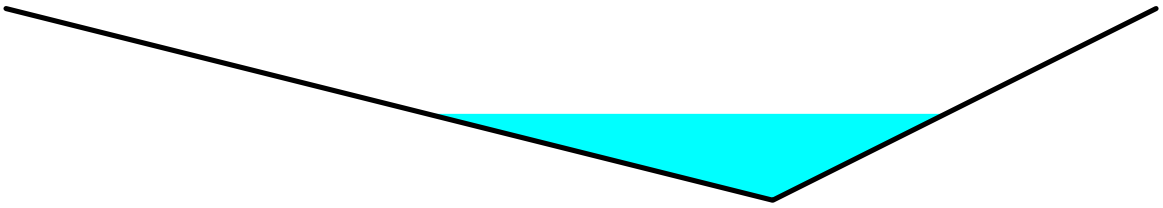
Summary for Reach D3: Southeast Ditch

Inflow Area = 2.661 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 16.74 cfs @ 11.90 hrs, Volume= 0.769 af
Outflow = 13.31 cfs @ 12.04 hrs, Volume= 0.769 af, Atten= 20%, Lag= 8.2 min
Routed to Pond SWB 3 : Storm Water Basin 3

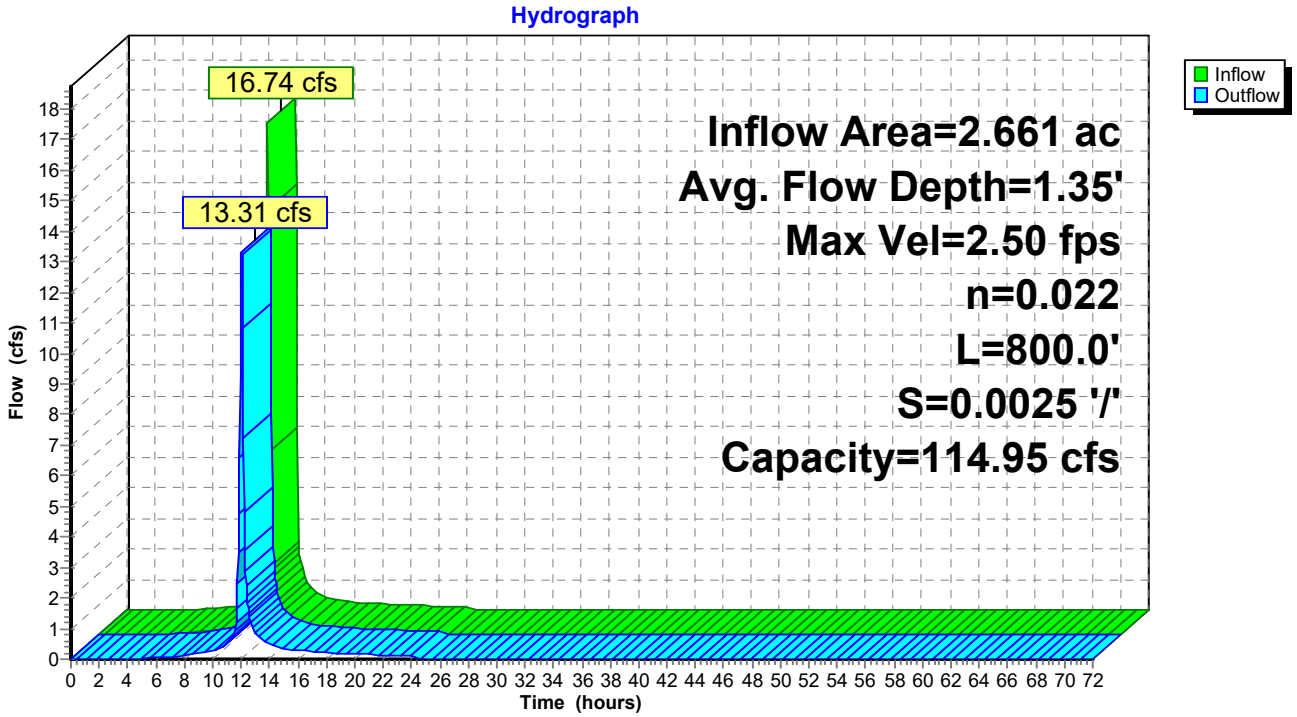
Routing by Stor-Ind+Trans method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.50 fps, Min. Travel Time= 5.3 min
Avg. Velocity = 0.75 fps, Avg. Travel Time= 17.9 min

Peak Storage= 4,379 cf @ 11.95 hrs
Average Depth at Peak Storage= 1.35' , Surface Width= 8.10'
Bank-Full Depth= 3.00' Flow Area= 27.0 sf, Capacity= 114.95 cfs

0.00' x 3.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 4.0 2.0 '/' Top Width= 18.00'
Length= 800.0' Slope= 0.0025 '/'
Inlet Invert= 1,228.00', Outlet Invert= 1,226.00'



Reach D3: Southeast Ditch

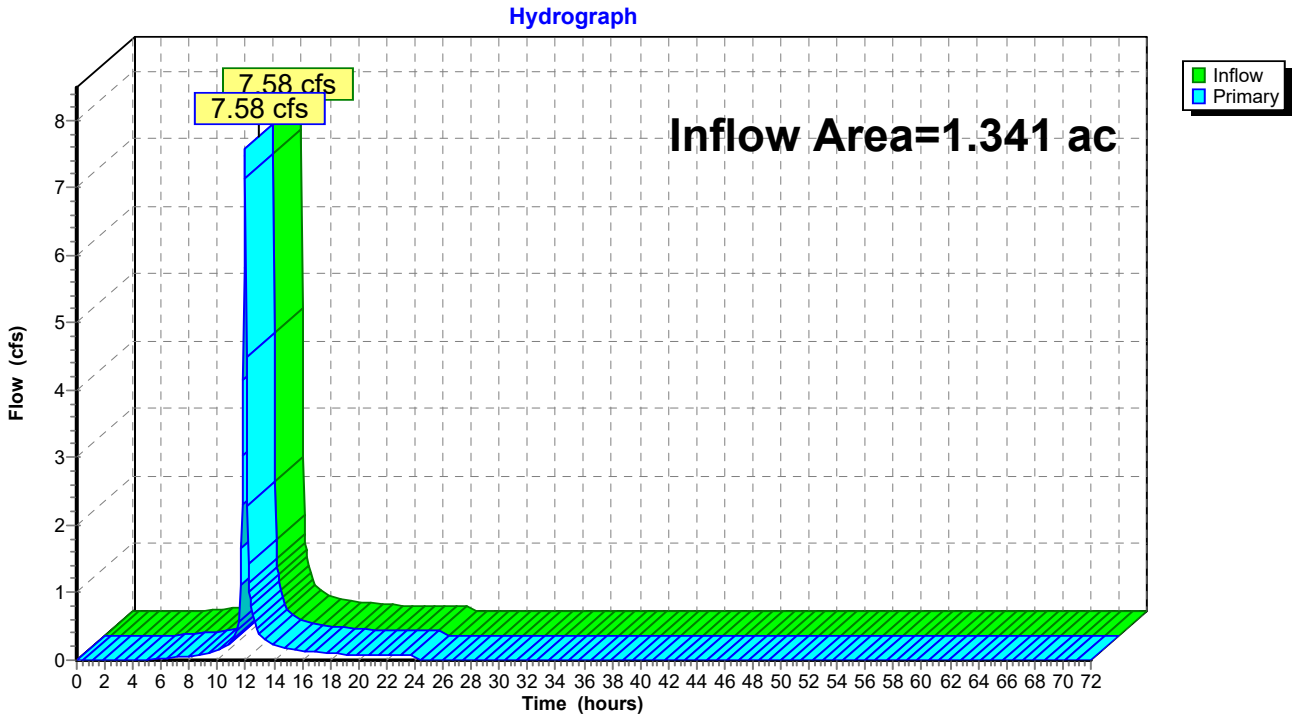


Summary for Pond SWB 1: Storm Water Basin 1

Inflow Area = 1.341 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 7.58 cfs @ 11.97 hrs, Volume= 0.388 af
Primary = 7.58 cfs @ 11.97 hrs, Volume= 0.388 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Pond SWB 1: Storm Water Basin 1

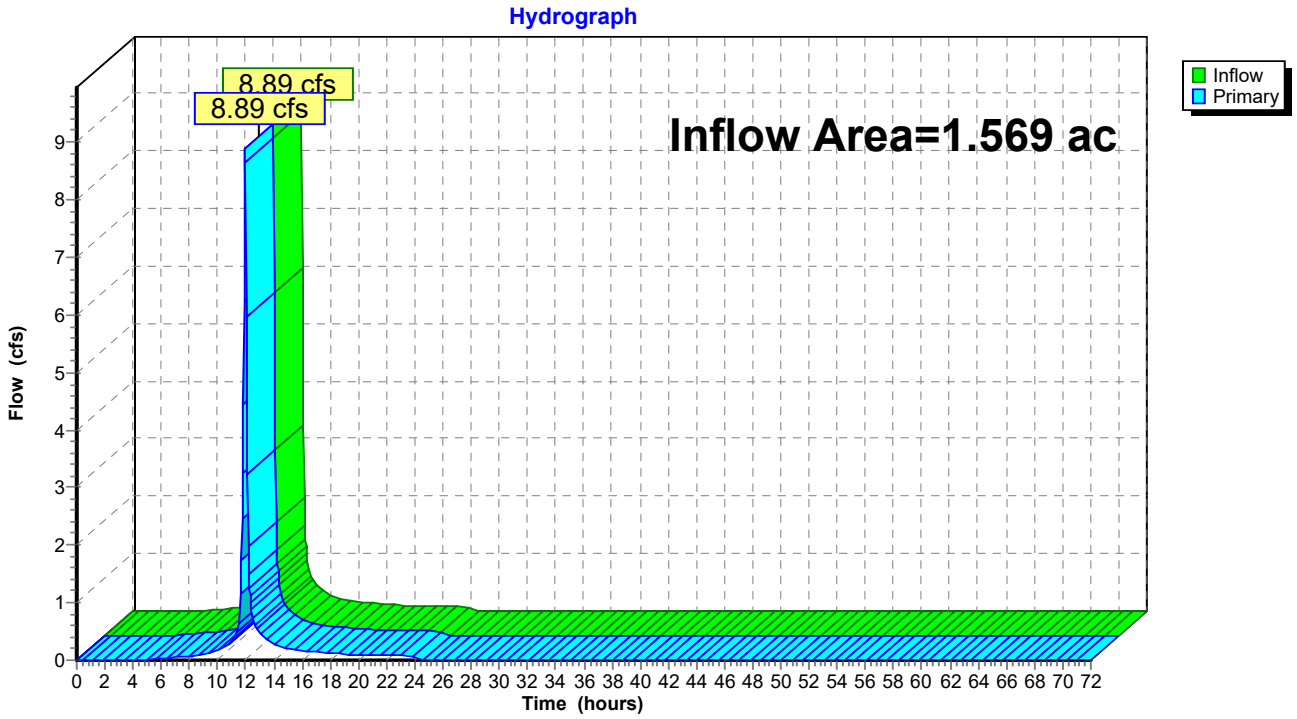


Summary for Pond SWB 2: Storm Water Basin 2

Inflow Area = 1.569 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 8.89 cfs @ 11.98 hrs, Volume= 0.453 af
Primary = 8.89 cfs @ 11.98 hrs, Volume= 0.453 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Pond SWB 2: Storm Water Basin 2

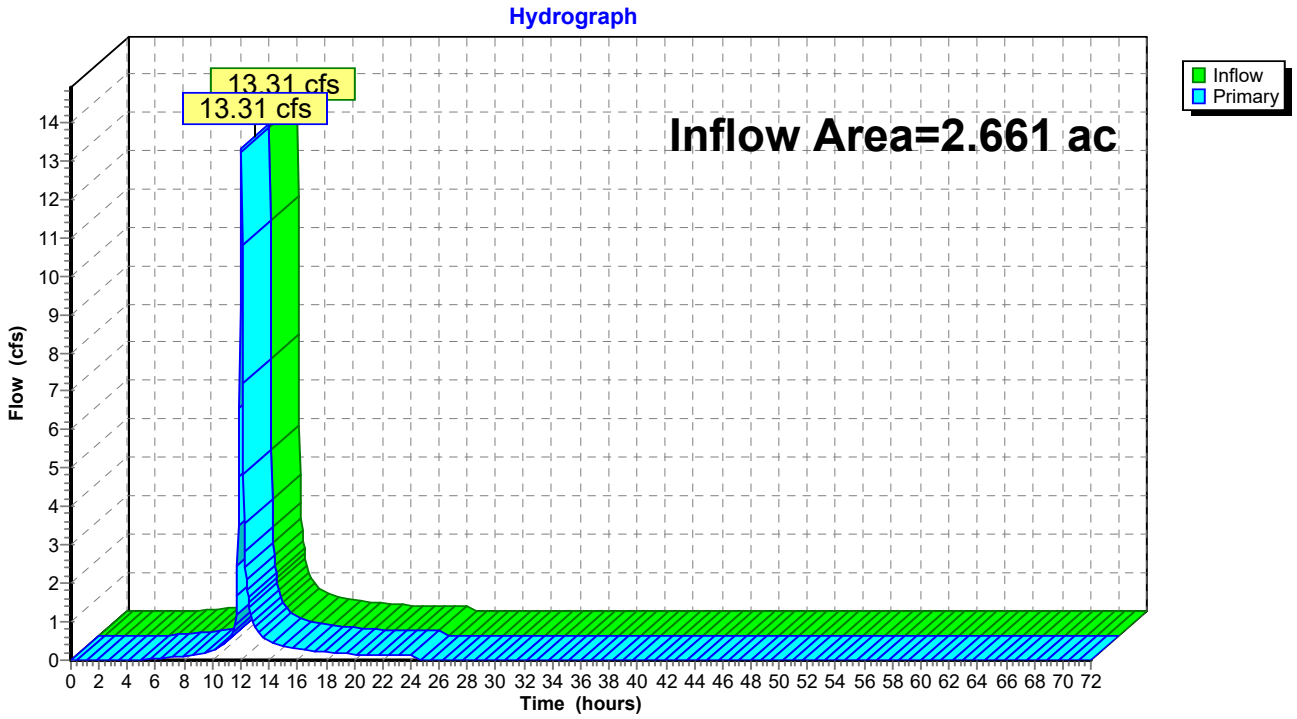


Summary for Pond SWB 3: Storm Water Basin 3

Inflow Area = 2.661 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 13.31 cfs @ 12.04 hrs, Volume= 0.769 af
Primary = 13.31 cfs @ 12.04 hrs, Volume= 0.769 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

Pond SWB 3: Storm Water Basin 3



Appendix D Stormwater Run-off Calculations

GEI Consultants	**Client**	WEC Energy Group			**Page**	1 of 2
	Project	WDS3 LF Run-on and Run-off Control Plan			**Rev.**	2
	By	R. Duncan	**Chk.**	A. Schwoerer	**App.**	J. Trast
	Date	10/22/2026	**Date**	10/23/2026	**Date**	10/26/2026
GEI Project No.	2103691	**Document No.**	N/A			
Subject	Stormwater Run-off Calculations					

Purpose:

The purpose of this calculation is to model the stormwater run-off associated with a 24-hour, 25-year precipitation event at Weston Disposal Site No. 3 (WDS3) from Cells 1, 2, and 3. In addition, this analysis was completed to confirm the current run-off control system for the construction of Cells 1, 2, and 3 can adequately manage the 24-hour, 25-year precipitation event.

Design Criteria and Assumptions:

1. The rainfall depth estimation for the 24-hour, 25-year precipitation event was determined to be 4.47 inches (included under a separate calculation package). The rainfall depth was determined by following procedures outlined in Precipitation-Frequency (PF) Atlas of the United States (Atlas 14, Volume 8, Version 2: Wisconsin).
2. The eastern, a portion of the northern, and a portion of the southern border of the landfill has been closed with final cover. Flow from this area moves along the perimeter berm to manholes that discharge into stormwater basins. This closed area totals 5.57 acres.
3. The landfill surface was modeled as bare ash, assuming hydrologic soil group C and a Runoff Curve Number (CN) of 91.
4. Perimeter ditches were modeled as 3-foot deep V-shaped channels with sides of 2H:1V on one side and 4H:1V on the other side. Perimeter ditch slopes ranged from 0.5% to 1%.
5. The size and geometry of the Cells 1, 2, and 3 ash slopes were obtained from Sheet C-1 from Appendix A of this report.
6. Stormwater on the active portion of the Cell was divided into 6 subcatchments and 4 stormwater ponding areas: north, northwest 1, northwest 2, southwest, mid-south, active Cell 3 ponding area, and stormwater ponding areas, as shown on Figure 1. Flow from all subcatchments will consist of sheet flow until it is collected by a conveyance channel at the toe of each slope. The main stormwater ponding area is SWP on the southwest corner of Cell 2. Ponding area NWP1, will have a 12-in diameter culvert allowing flow to ponding area NWP2, which will have a 12-in diameter culvert flowing to ponding area SWP. Ponding area C3P does not have culverts to any other ponding areas. In all four stormwater surge areas (NWP1, NWP2, SWP, and C3P), the water infiltrates into leachate collection granular drainage layer and is treated as leachate. Stormwater subcatchments and the stormwater surge areas are shown on Figure 1.
7. HydroCAD 10.0 was used to model the stormwater associated with the Weston Disposal Site No. 3 Ash Landfill.
8. Subcatchment, reach, and detention parameters are included in the attached HydroCAD Report.

Results:

In general, stormwater is conveyed off the slopes of Cells 1 and 2 as sheet flow until it is intercepted by the perimeter berm. The perimeter berms are 3-feet-deep and have a 4H:1V exterior slope and 2H:1V interior side slope. The exterior slope of the berm is the final cover of the cell. The interior slope is the perimeter berm.

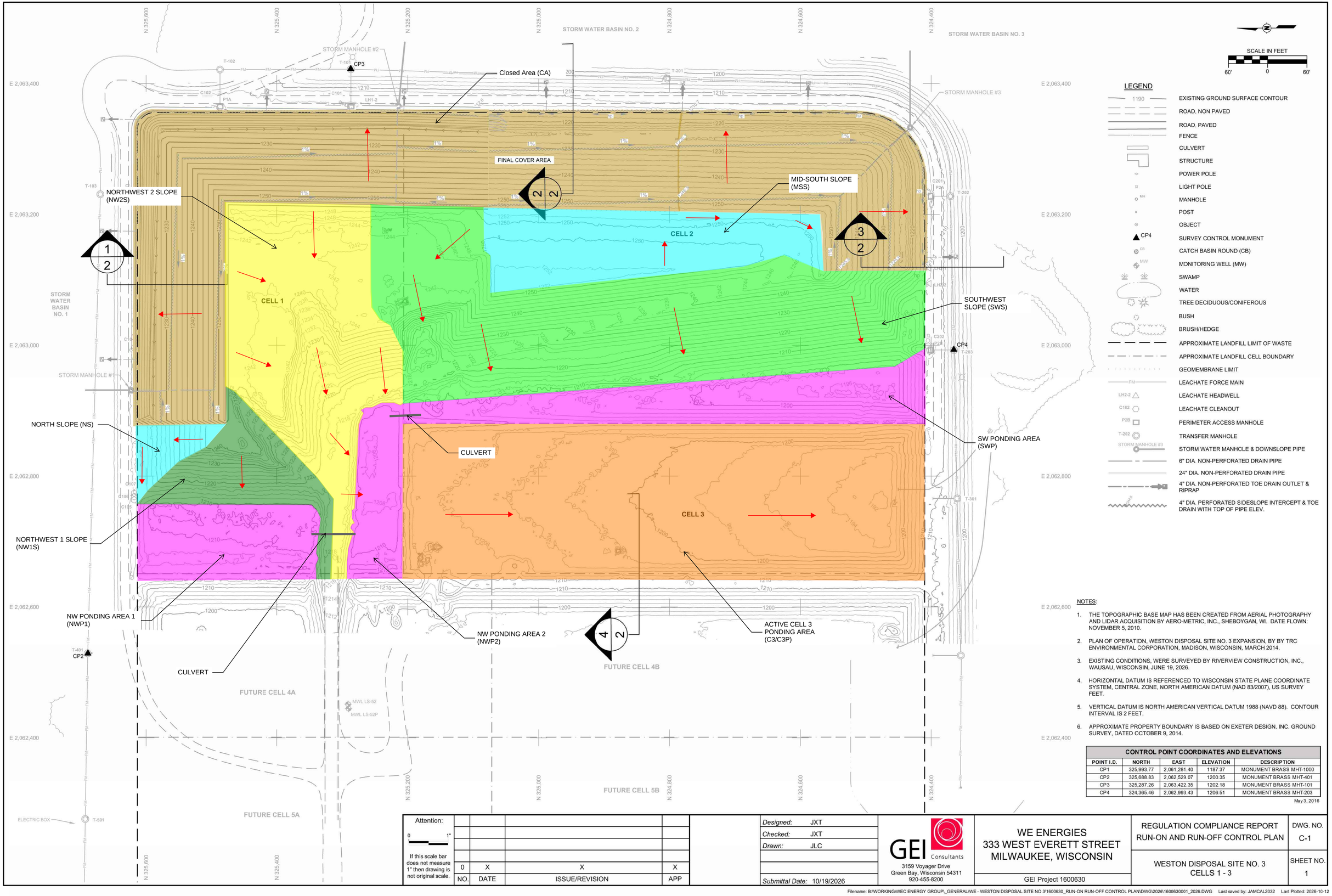
GEI Consultants	**Client**	WEC Energy Group			**Page**	2 of 2
	Project	WDS3 LF Run-on and Run-off Control Plan			**Rev.**	2
	By	R. Duncan	**Chk.**	A. Schwoerer	**App.**	J. Trast
	Date	10/22/2026	**Date**	10/23/2026	**Date**	10/26/2026
GEI Project No.	2103691	**Document No.**	N/A			
Subject	Stormwater Run-off Calculations					

The results of the stormwater modeling calculations indicate that the perimeter ditches located along Cells 1 and 2 are able to contain and convey the flow of runoff resulting from the 25-year, 24-hour precipitation event, and route it to one of the four ponding areas. The table below shows the assumed starting elevations, the maximum ponding elevations, and amount of freeboard anticipated following the 25-year, 24-hour storm.

Stormwater Surge Area	Starting Elevation (ft)	Max Elevation (ft)	Top of Pond Elevation (ft)	Freeboard (ft)
NW Ponding Area 1, NWP1	1209.00	1210.33	1212.00	1.67
NW Ponding Area 2, NWP2	1208.00	1209.43	1210.00	0.57
SW Ponding Area, SWP	1194.00	1202.81	1208.00	5.19
Active Cell 3 Ponding Area, C3P	1192.00	1196.37	1206.00	9.63

Attachments:

- Figure 1 –Stormwater Conveyance Diagram
- HydroCAD Summary Report



NOTES:

1. THE TOPOGRAPHIC BASE MAP HAS BEEN CREATED FROM AERIAL PHOTOGRAPHY AND LIDAR ACQUISITION BY AERO-METRIC, INC., SHEBOYGAN, WI. DATE FLOWN: NOVEMBER 5, 2010.
2. PLAN OF OPERATION, WESTON DISPOSAL SITE NO. 3 EXPANSION, BY BY TRC ENVIRONMENTAL CORPORATION, MADISON, WISCONSIN, MARCH 2014.
3. EXISTING CONDITIONS, WERE SURVEYED BY RIVERVIEW CONSTRUCTION, INC., WAUSAU, WISCONSIN, JUNE 19, 2026.
4. HORIZONTAL DATUM IS REFERENCED TO WISCONSIN STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE, NORTH AMERICAN DATUM (NAD 83/2007), US SURVEY FEET.
5. VERTICAL DATUM IS NORTH AMERICAN VERTICAL DATUM 1988 (NAVD 88). CONTOUR INTERVAL IS 2 FEET.
6. APPROXIMATE PROPERTY BOUNDARY IS BASED ON EXETER DESIGN, INC. GROUND SURVEY, DATED OCTOBER 9, 2014.

CONTROL POINT COORDINATES AND ELEVATIONS

POINT I.D.	NORTH	EAST	ELEVATION	DESCRIPTION
CP1	325,993.77	2,061,281.40	1187.37	MONUMENT BRASS MHT-1000
CP2	325,688.83	2,062,529.07	1200.35	MONUMENT BRASS MHT-401
CP3	325,287.26	2,063,422.35	1202.18	MONUMENT BRASS MHT-101
CP4	324,365.46	2,062,993.43	1206.51	MONUMENT BRASS MHT-203

May 3, 2016

Attention:

0 1"

If this scale bar does not measure 1" then drawing is not original scale.

NO.	DATE	ISSUE/REVISION	APP
0	X	X	X

Designed: JXT
Checked: JXT
Drawn: JLC
Submittal Date: 10/19/2026

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Green Bay, Wisconsin 54311
920-455-8200

WE ENERGIES
333 WEST EVERETT STREET
MILWAUKEE, WISCONSIN

GEI Project 1600630

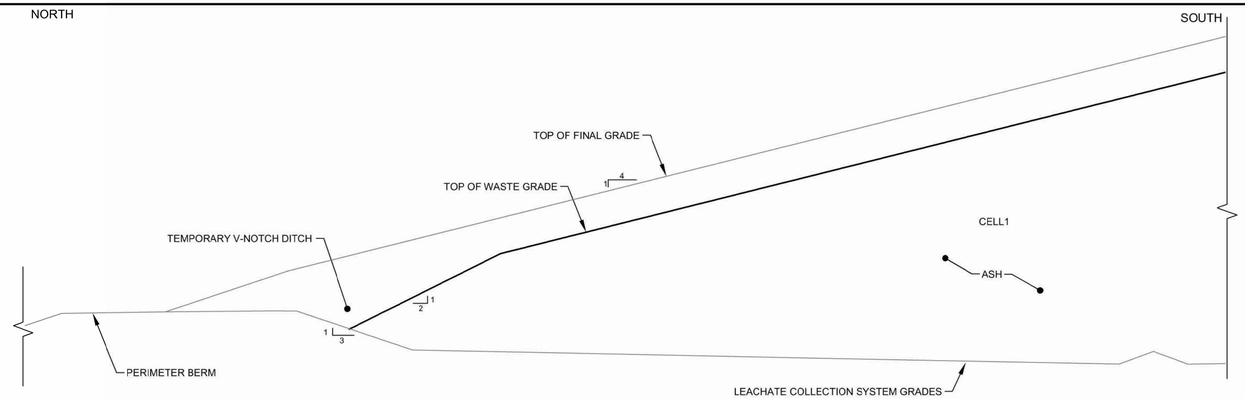
REGULATION COMPLIANCE REPORT
RUN-ON AND RUN-OFF CONTROL PLAN

DWG. NO. C-1

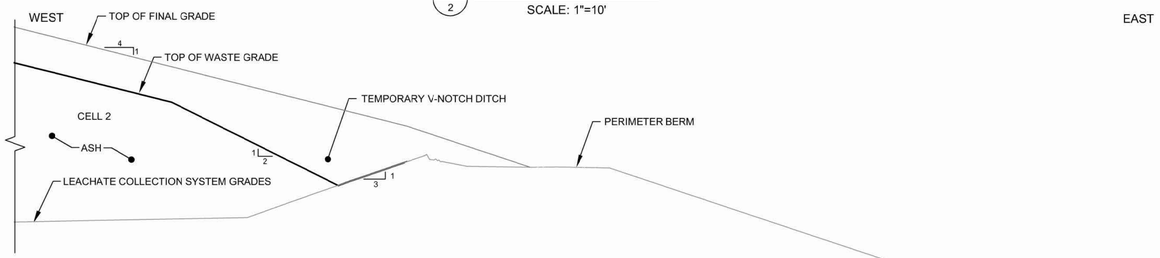
WESTON DISPOSAL SITE NO. 3
CELLS 1 - 3

SHEET NO. 1

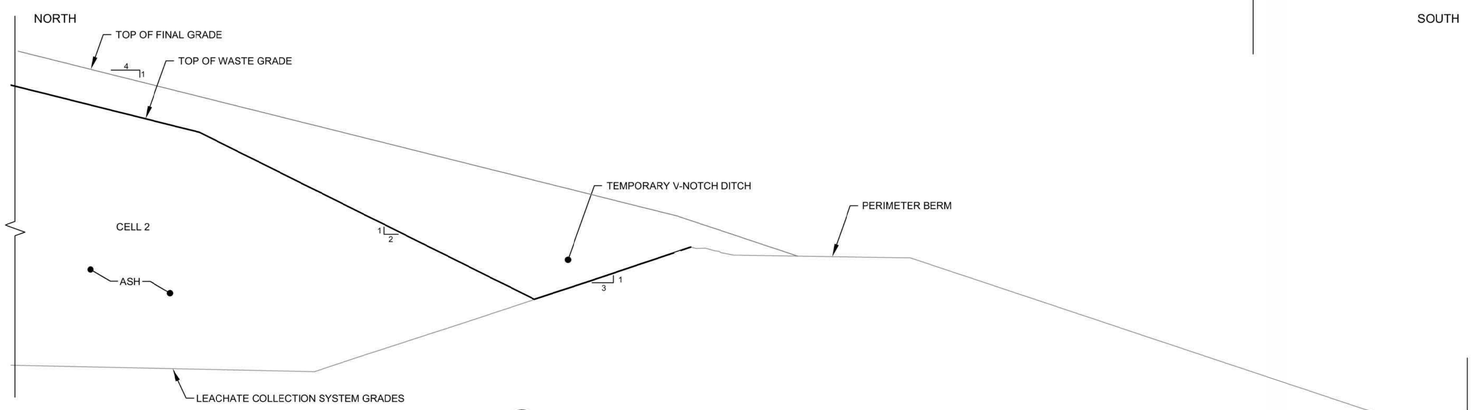
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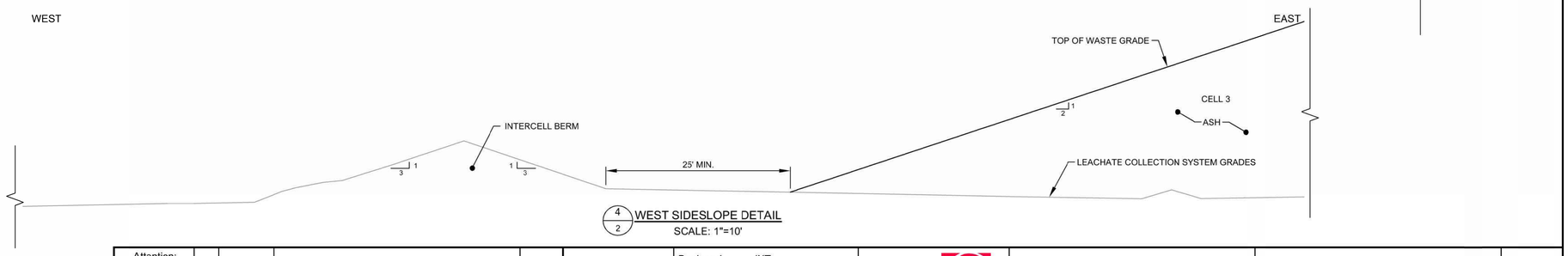
1/2 NORTH SIDESLOPE DETAIL
SCALE: 1"=10'



2/2 EAST SIDESLOPE DETAIL
SCALE: 1"=10'



3/2 SOUTH SIDESLOPE DETAIL
SCALE: 1"=10'



4/2 WEST SIDESLOPE DETAIL
SCALE: 1"=10'

Attention:

0 1"

If this scale bar does not measure 1" then drawing is not original scale.

NO.	DATE	ISSUE/REVISION	APP
0	X	X	X

Designed:	JXT
Checked:	JXT
Drawn:	JLC
Submittal Date:	10/19/2026

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333 WEST EVERETT STREET
MILWAUKEE, WISCONSIN

GEI Project 1600630

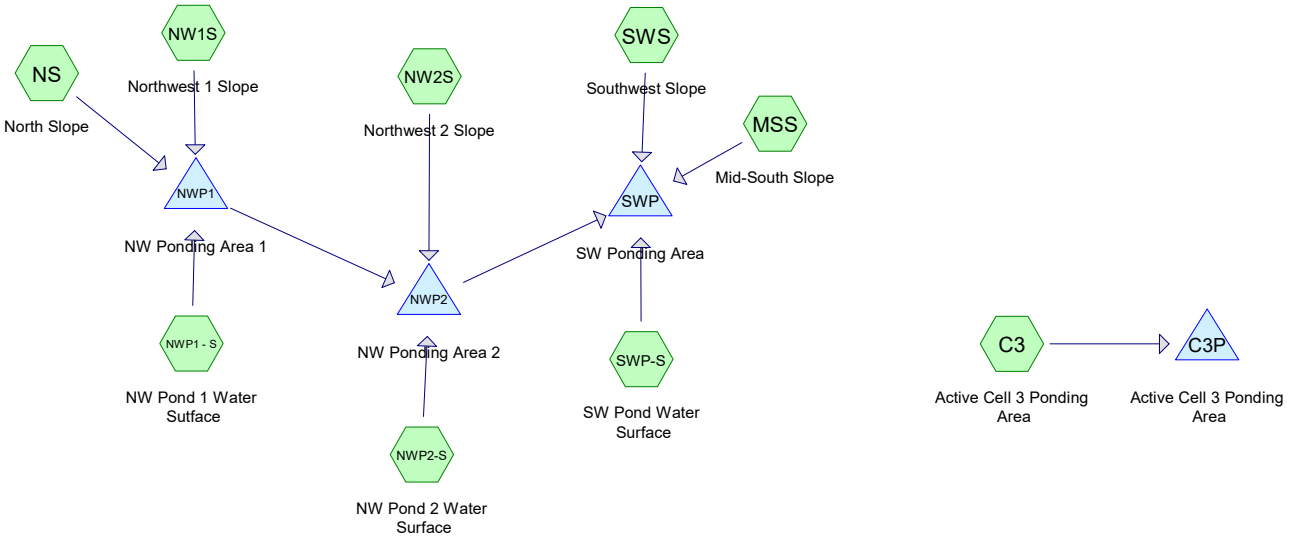
REGULATION COMPLIANCE REPORT
RUN-ON AND RUN-OFF CONTROL PLAN

DWG. NO. C-2

WESTON DISPOSAL SITE NO. 3
CELLS 1 - 3 DETAILS

SHEET NO. 2

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Subcat Reach Pond Link

Routing Diagram for Runoff_2026
Prepared by GEI Consultants, Printed 10/22/2026
HydroCAD® 10.20-9a s/n 01250 © 2026 HydroCAD Software Solutions LLC

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
12.110	91	Newly graded area, HSG C (C3, MSS, NS, NW1S, NW2S, SWS)
2.215	98	Water Surface, HSG C (NWP1 - S, NWP2-S, SWP-S)
14.325	**92**	**TOTAL AREA**

Soil Listing (all nodes)

Area (acres)	Soil Group	Subcatchment Numbers
0.000	HSG A	
0.000	HSG B	
14.325	HSG C	C3, MSS, NS, NW1S, NW2S, NWP1 - S, NWP2-S, SWP-S, SWS
0.000	HSG D	
0.000	Other	
14.325		**TOTAL AREA**

Ground Covers (all nodes)

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
0.000	0.000	12.110	0.000	0.000	12.110	Newly graded area	C3, MSS, NS, NW1S, NW2S, SWS
0.000	0.000	2.215	0.000	0.000	2.215	Water Surface	NWP1 - S, NWP2-S, SWP-S
0.000	**0.000**	**14.325**	**0.000**	**0.000**	**14.325**	**TOTAL AREA**	

Time span=0.00-72.00 hrs, dt=0.05 hrs, 1441 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment C3: Active Cell 3 Ponding Runoff Area=4.380 ac 0.00% Impervious Runoff Depth=3.47"
Flow Length=725' Tc=7.6 min CN=91 Runoff=23.64 cfs 1.266 af

Subcatchment MSS: Mid-South Slope Runoff Area=1.170 ac 0.00% Impervious Runoff Depth=3.47"
Flow Length=85' Slope=0.0360 '/' Tc=0.9 min CN=91 Runoff=7.53 cfs 0.338 af

Subcatchment NS: North Slope Runoff Area=0.190 ac 0.00% Impervious Runoff Depth=3.47"
Flow Length=100' Slope=0.2580 '/' Tc=0.5 min CN=91 Runoff=1.24 cfs 0.055 af

Subcatchment NW1S: Northwest 1 Slope Runoff Area=0.640 ac 0.00% Impervious Runoff Depth=3.47"
Flow Length=190' Slope=0.1160 '/' Tc=1.1 min CN=91 Runoff=4.08 cfs 0.185 af

Subcatchment NW2S: Northwest 2 Slope Runoff Area=2.140 ac 0.00% Impervious Runoff Depth=3.47"
Flow Length=212' Slope=0.1170 '/' Tc=1.2 min CN=91 Runoff=13.59 cfs 0.619 af

Subcatchment NWP1 - S: NW Pond 1 Runoff Area=25,886 sf 100.00% Impervious Runoff Depth=4.23"
Tc=0.0 min CN=98 Runoff=4.24 cfs 0.210 af

Subcatchment NWP2-S: NW Pond 2 Runoff Area=14,566 sf 100.00% Impervious Runoff Depth=4.23"
Tc=0.0 min CN=98 Runoff=2.39 cfs 0.118 af

Subcatchment SWP-S: SW Pond Water Runoff Area=56,040 sf 100.00% Impervious Runoff Depth=4.23"
Tc=0.0 min CN=98 Runoff=9.19 cfs 0.454 af

Subcatchment SWS: Southwest Slope Runoff Area=3.590 ac 0.00% Impervious Runoff Depth=3.47"
Flow Length=300' Slope=0.0500 '/' Tc=2.2 min CN=91 Runoff=21.80 cfs 1.038 af

Pond C3P: Active Cell 3 Ponding Area Peak Elev=1,196.37' Storage=55,139 cf Inflow=23.64 cfs 1.266 af
Outflow=0.00 cfs 0.000 af

Pond NWP1: NW Ponding Area 1 Peak Elev=1,210.33' Storage=12,902 cf Inflow=9.53 cfs 0.450 af
12.0" Round Culvert n=0.013 L=65.0' S=0.0308 '/' Outflow=0.45 cfs 0.259 af

Pond NWP2: NW Ponding Area 2 Peak Elev=1,209.43' Storage=11,219 cf Inflow=15.99 cfs 0.995 af
12.0" Round Culvert n=0.013 L=50.0' S=0.1000 '/' Outflow=3.64 cfs 0.995 af

Pond SWP: SW Ponding Area Peak Elev=1,202.81' Storage=123,047 cf Inflow=41.13 cfs 2.825 af
Outflow=0.00 cfs 0.000 af

Total Runoff Area = 14.325 ac Runoff Volume = 4.282 af Average Runoff Depth = 3.59"
84.54% Pervious = 12.110 ac 15.46% Impervious = 2.215 ac

Summary for Subcatchment C3: Active Cell 3 Ponding Area

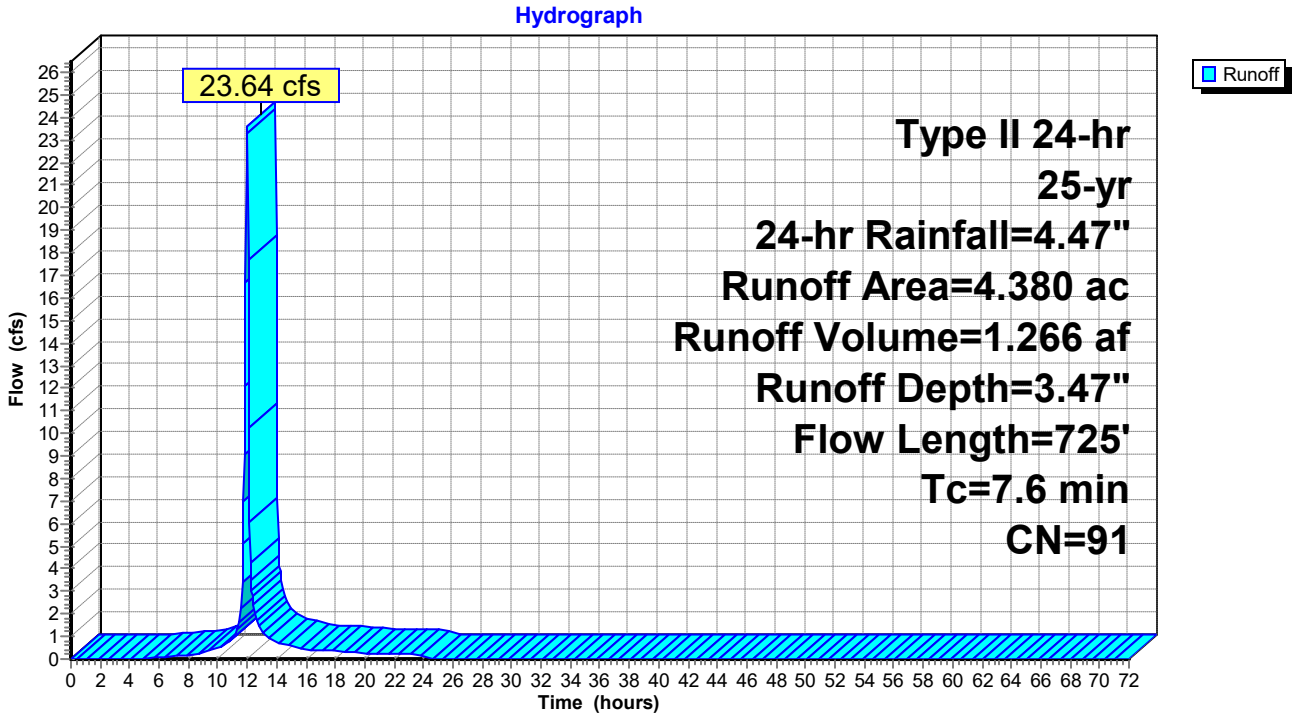
Runoff = 23.64 cfs @ 11.99 hrs, Volume= 1.266 af, Depth= 3.47"
Routed to Pond C3P : Active Cell 3 Ponding Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (ac)	CN	Description
4.380	91	Newly graded area, HSG C
4.380		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.7	300	0.0300	1.82		**Sheet Flow, Sheet Flow** Smooth surfaces n= 0.011 P2= 2.61"
4.9	425	0.0211	1.45		**Shallow Concentrated Flow, Shallow Flow** Nearly Bare & Untilled Kv= 10.0 fps
7.6	725	Total			

Subcatchment C3: Active Cell 3 Ponding Area



Summary for Subcatchment MSS: Mid-South Slope

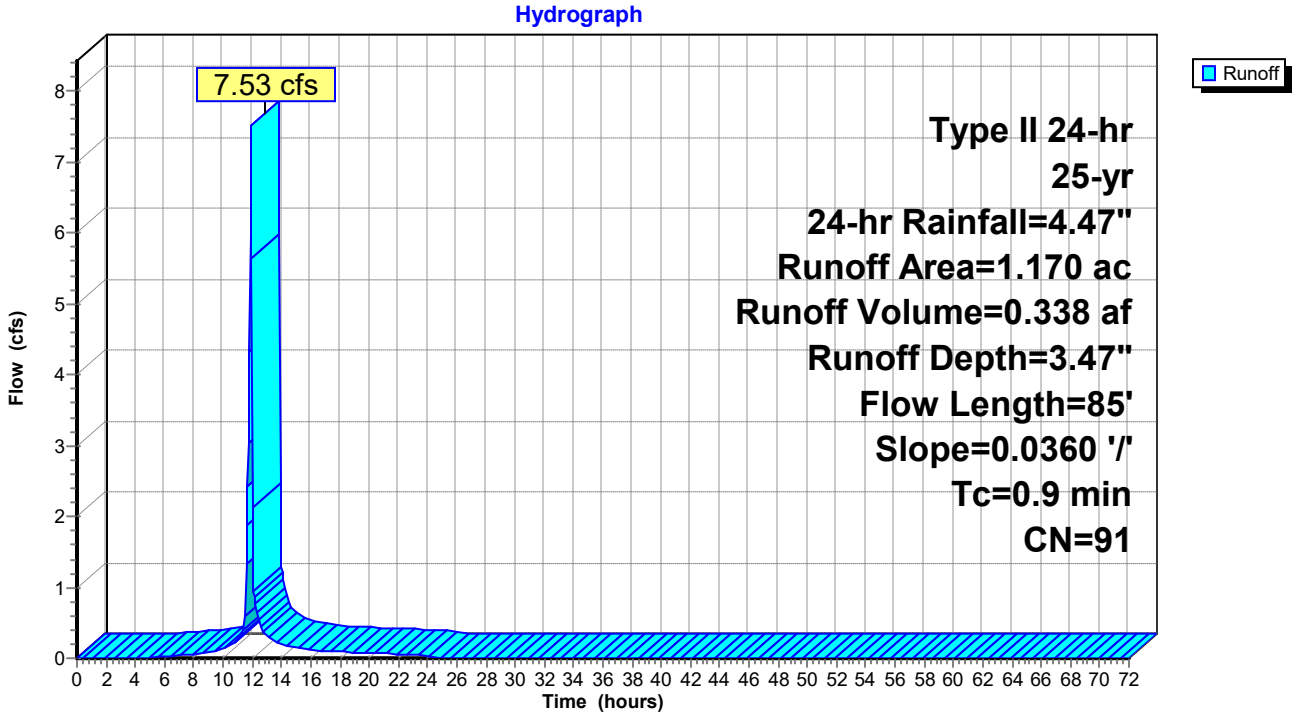
Runoff = 7.53 cfs @ 11.90 hrs, Volume= 0.338 af, Depth= 3.47"
Routed to Pond SWP : SW Ponding Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (ac)	CN	Description
1.170	91	Newly graded area, HSG C
1.170		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.9	85	0.0360	1.52		**Sheet Flow, Sheet Flow** Smooth surfaces n= 0.011 P2= 2.61"

Subcatchment MSS: Mid-South Slope



Summary for Subcatchment NS: North Slope

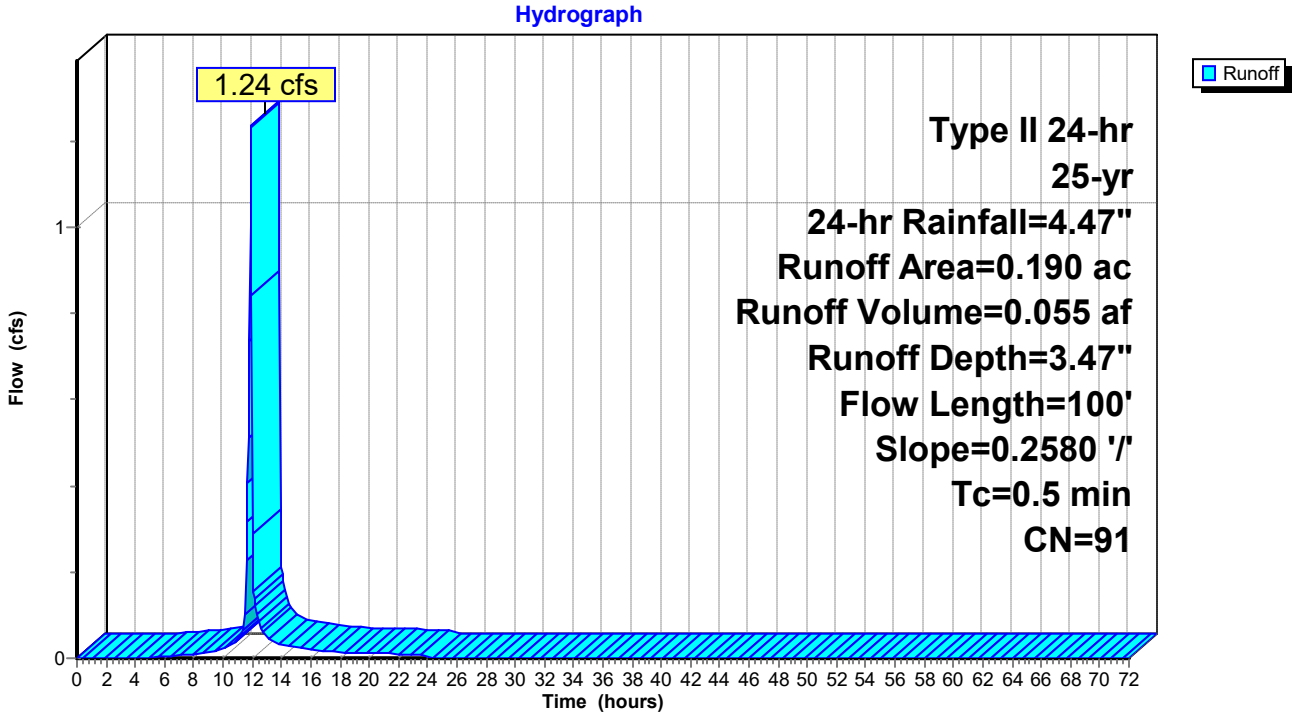
Runoff = 1.24 cfs @ 11.89 hrs, Volume= 0.055 af, Depth= 3.47"
Routed to Pond NWP1 : NW Ponding Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (ac)	CN	Description
0.190	91	Newly graded area, HSG C
0.190		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
0.5	100	0.2580	3.46		**Sheet Flow, Sheet Flow** Smooth surfaces n= 0.011 P2= 2.61"

Subcatchment NS: North Slope



Summary for Subcatchment NW1S: Northwest 1 Slope

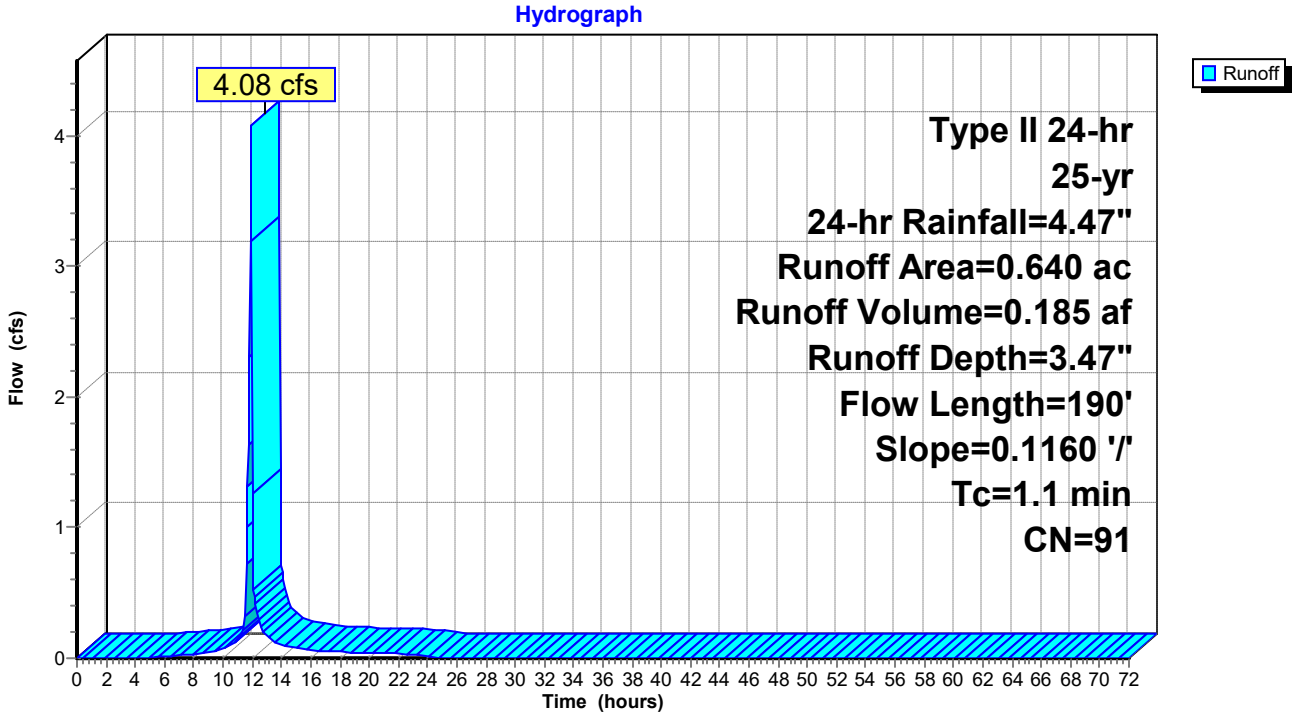
Runoff = 4.08 cfs @ 11.90 hrs, Volume= 0.185 af, Depth= 3.47"
Routed to Pond NWP1 : NW Ponding Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (ac)	CN	Description
0.640	91	Newly graded area, HSG C
0.640		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.1	190	0.1160	2.85		**Sheet Flow, Sheet Flow** Smooth surfaces n= 0.011 P2= 2.61"

Subcatchment NW1S: Northwest 1 Slope



Summary for Subcatchment NW2S: Northwest 2 Slope

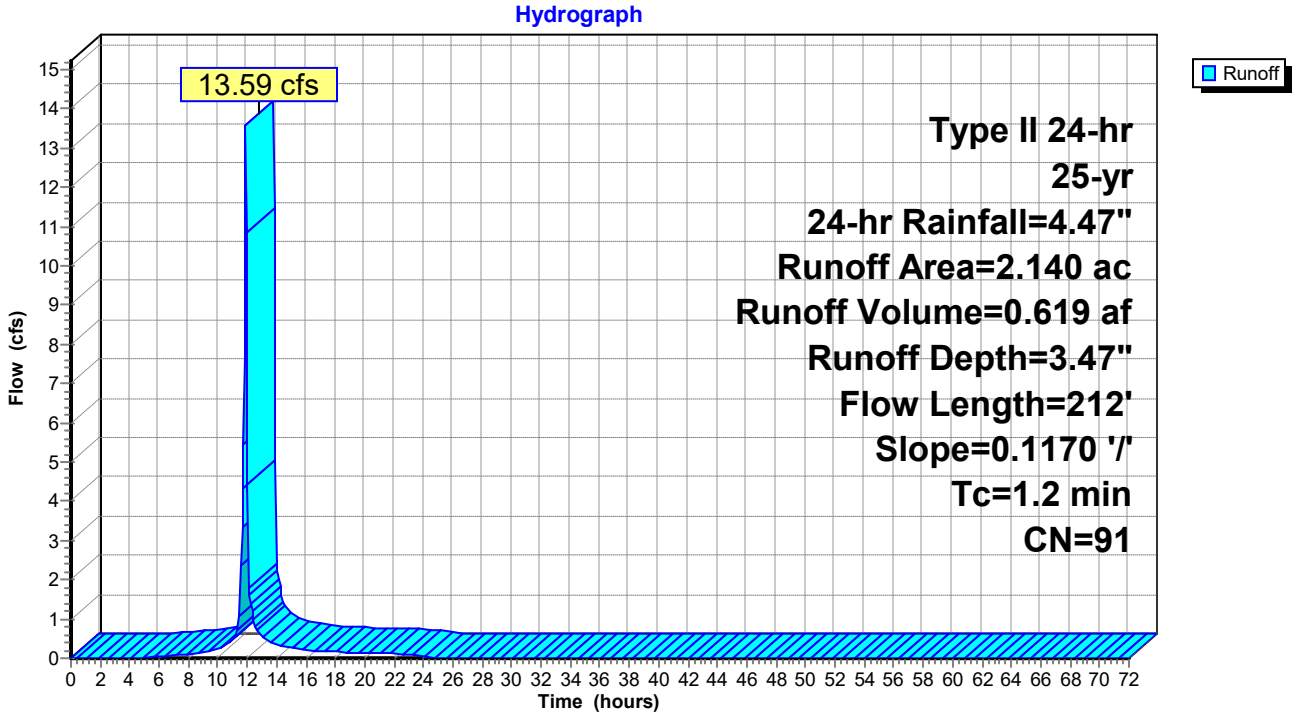
Runoff = 13.59 cfs @ 11.90 hrs, Volume= 0.619 af, Depth= 3.47"
Routed to Pond NWP2 : NW Ponding Area 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (ac)	CN	Description
2.140	91	Newly graded area, HSG C
2.140		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.2	212	0.1170	2.93		**Sheet Flow, Sheet Flow** Smooth surfaces n= 0.011 P2= 2.61"

Subcatchment NW2S: Northwest 2 Slope



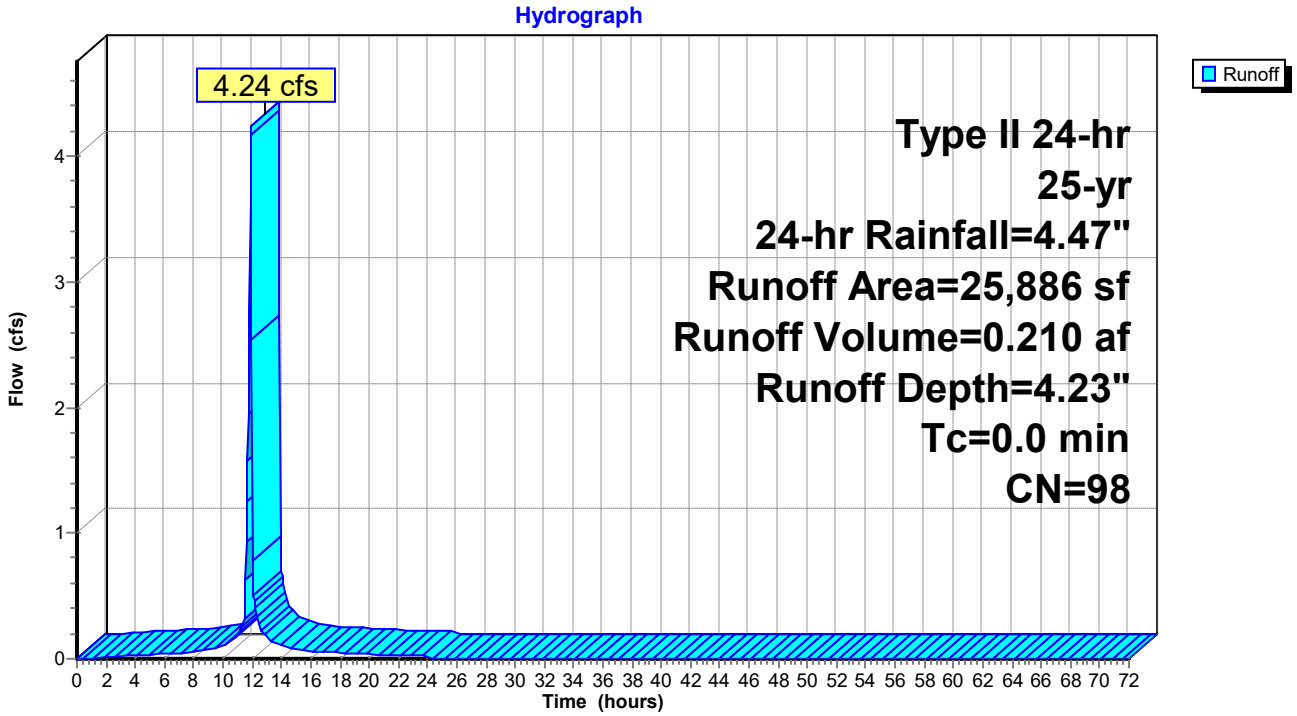
Summary for Subcatchment NWP1 - S: NW Pond 1 Water Sutface

Runoff = 4.24 cfs @ 11.89 hrs, Volume= 0.210 af, Depth= 4.23"
Routed to Pond NWP1 : NW Ponding Area 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (sf)	CN	Description
25,886	98	Water Surface, HSG C
25,886		100.00% Impervious Area

Subcatchment NWP1 - S: NW Pond 1 Water Sutface



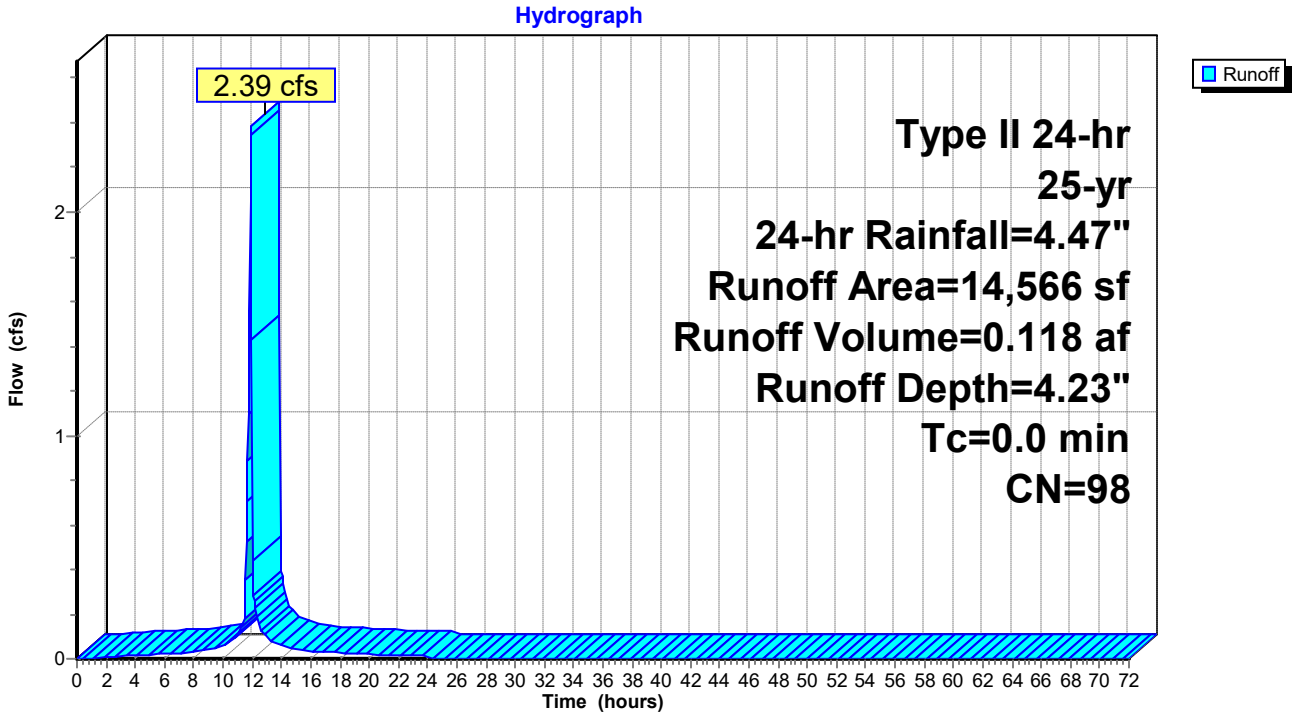
Summary for Subcatchment NWP2-S: NW Pond 2 Water Surface

Runoff = 2.39 cfs @ 11.89 hrs, Volume= 0.118 af, Depth= 4.23"
Routed to Pond NWP2 : NW Ponding Area 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (sf)	CN	Description
14,566	98	Water Surface, HSG C
14,566		100.00% Impervious Area

Subcatchment NWP2-S: NW Pond 2 Water Surface



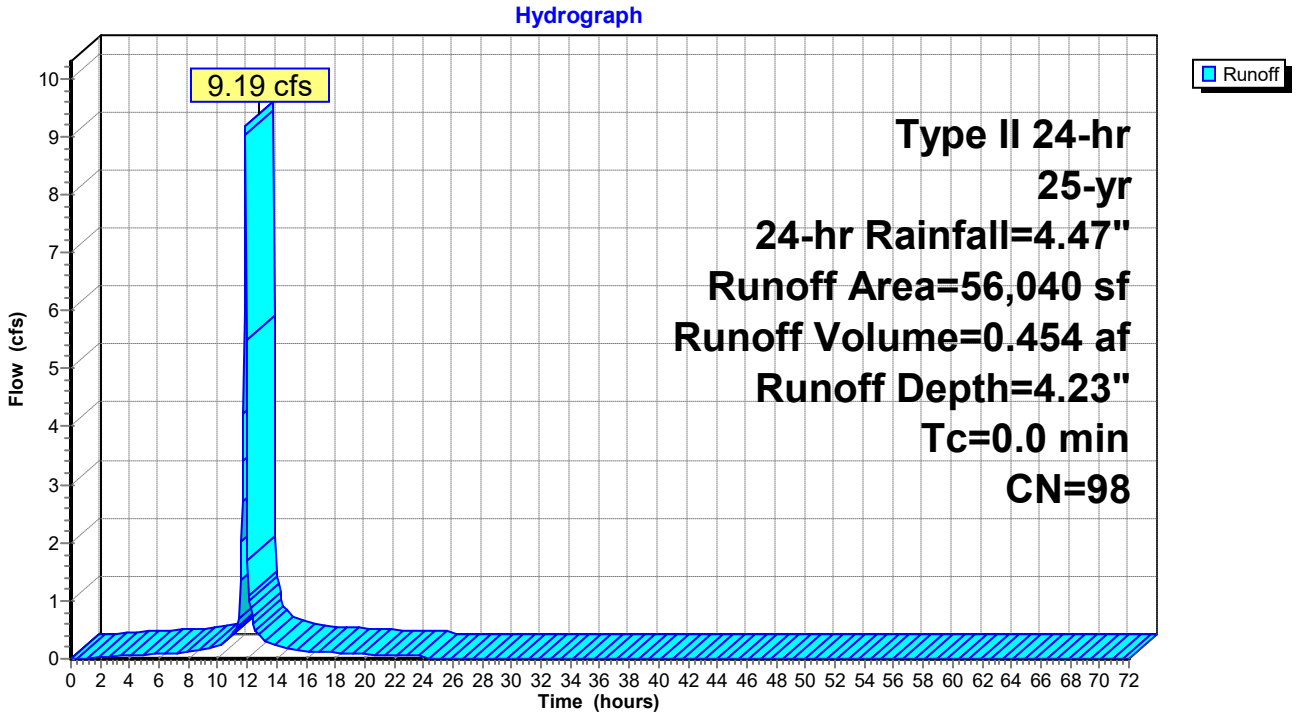
Summary for Subcatchment SWP-S: SW Pond Water Surface

Runoff = 9.19 cfs @ 11.89 hrs, Volume= 0.454 af, Depth= 4.23"
Routed to Pond SWP : SW Ponding Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (sf)	CN	Description
56,040	98	Water Surface, HSG C
56,040		100.00% Impervious Area

Subcatchment SWP-S: SW Pond Water Surface



Summary for Subcatchment SWS: Southwest Slope

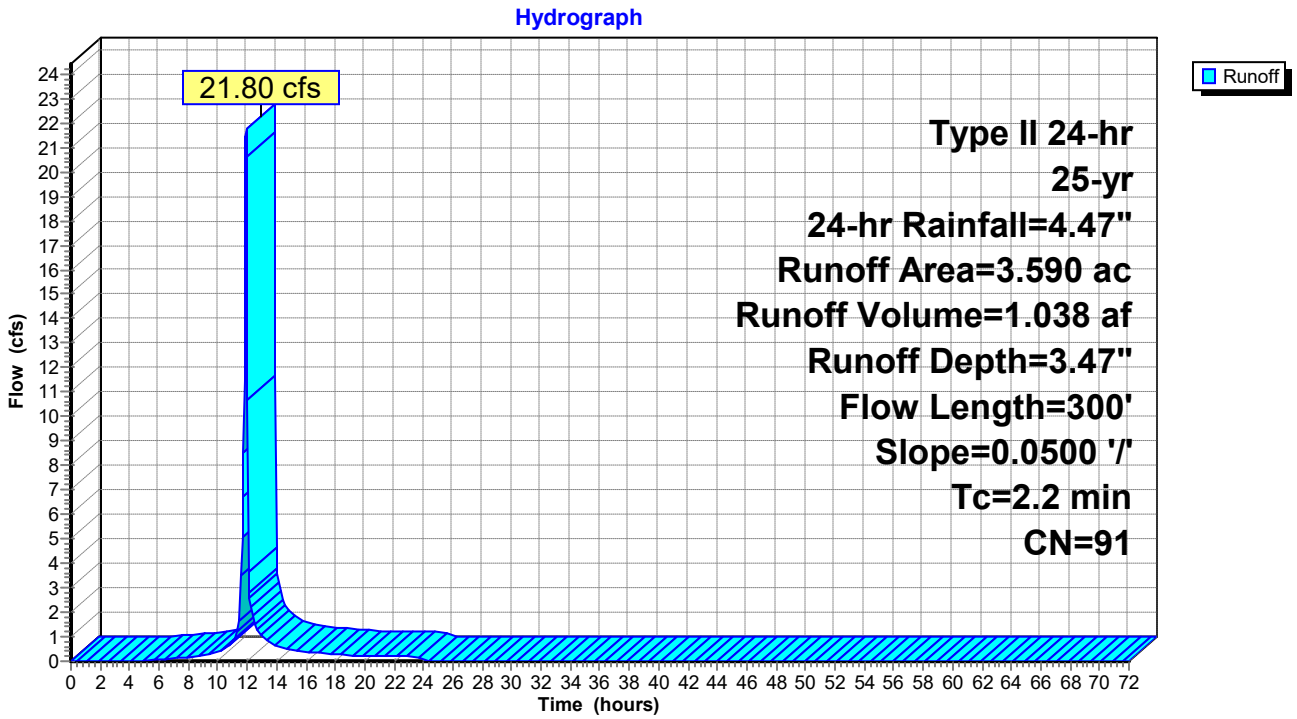
Runoff = 21.80 cfs @ 11.92 hrs, Volume= 1.038 af, Depth= 3.47"
Routed to Pond SWP : SW Ponding Area

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Type II 24-hr 25-yr, 24-hr Rainfall=4.47"

Area (ac)	CN	Description
3.590	91	Newly graded area, HSG C
3.590		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
2.2	300	0.0500	2.23		**Sheet Flow, Sheet Flow** Smooth surfaces n= 0.011 P2= 2.61"

Subcatchment SWS: Southwest Slope



Summary for Pond C3P: Active Cell 3 Ponding Area

Inflow Area = 4.380 ac, 0.00% Impervious, Inflow Depth = 3.47" for 25-yr, 24-hr event
Inflow = 23.64 cfs @ 11.99 hrs, Volume= 1.266 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

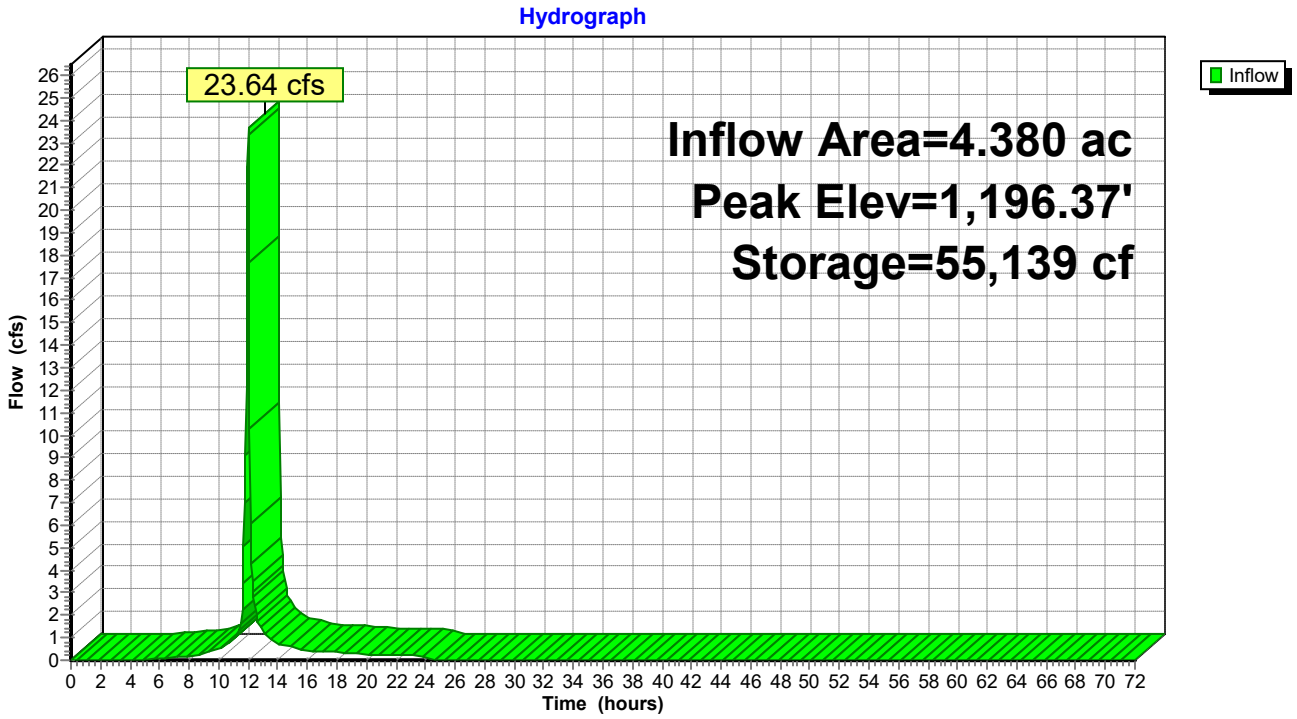
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 1,196.37' @ 24.50 hrs Surf.Area= 29,800 sf Storage= 55,139 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	1,192.00'	833,089 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,192.00	487	0	0
1,194.00	9,000	9,487	9,487
1,196.00	26,166	35,166	44,653
1,198.00	45,559	71,725	116,378
1,200.00	63,720	109,279	225,657
1,202.00	88,006	151,726	377,383
1,204.00	113,035	201,041	578,424
1,206.00	141,630	254,665	833,089

Pond C3P: Active Cell 3 Ponding Area



Summary for Pond NWP1: NW Ponding Area 1

Inflow Area = 1.424 ac, 41.72% Impervious, Inflow Depth = 3.79" for 25-yr, 24-hr event
Inflow = 9.53 cfs @ 11.89 hrs, Volume= 0.450 af
Outflow = 0.45 cfs @ 12.80 hrs, Volume= 0.259 af, Atten= 95%, Lag= 54.1 min
Primary = 0.45 cfs @ 12.80 hrs, Volume= 0.259 af
Routed to Pond NWP2 : NW Ponding Area 2

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 1,210.33' @ 12.80 hrs Surf.Area= 15,796 sf Storage= 12,902 cf

Plug-Flow detention time= 476.6 min calculated for 0.259 af (58% of inflow)
Center-of-Mass det. time= 366.1 min (1,129.0 - 762.9)

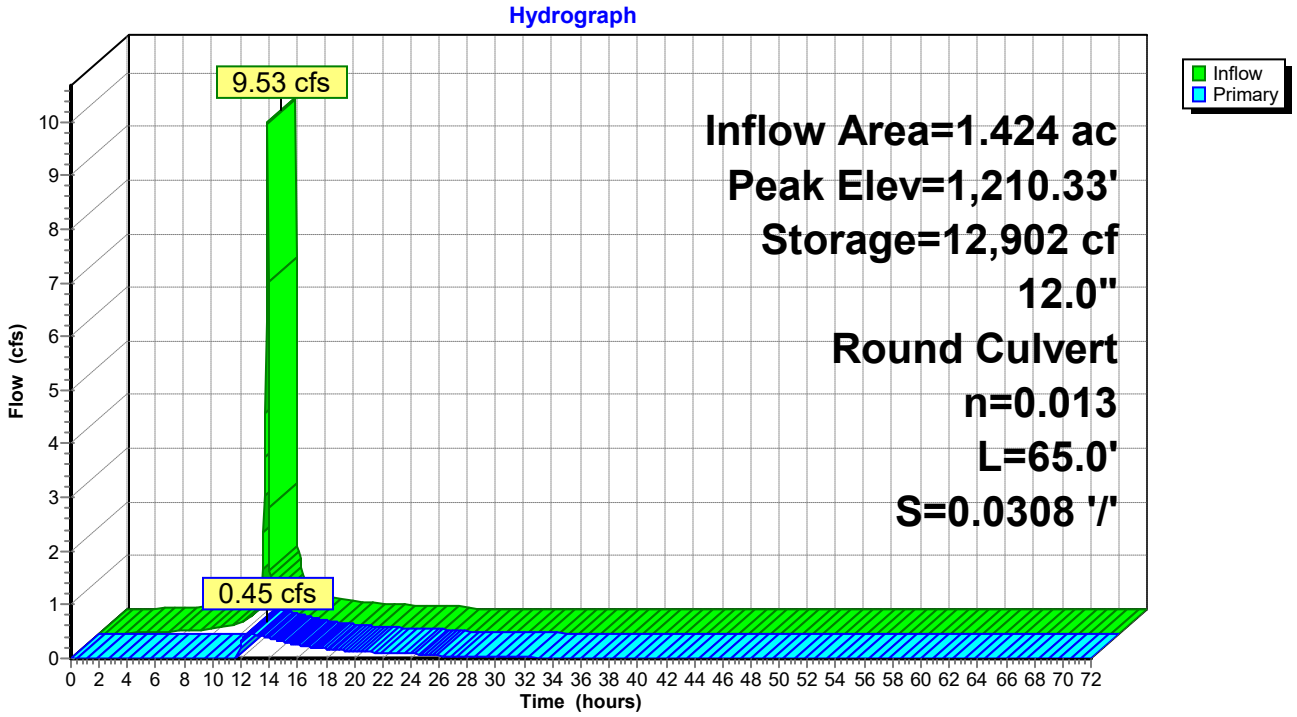
Volume	Invert	Avail.Storage	Storage Description
#1	1,209.00'	49,170 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,209.00	3,389	0	0
1,210.00	12,897	8,143	8,143
1,211.00	21,635	17,266	25,409
1,212.00	25,886	23,761	49,170

Device	Routing	Invert	Outlet Devices
#1	Primary	1,210.00'	**12.0" Round Culvert** L= 65.0' Ke= 0.500 Inlet / Outlet Invert= 1,210.00' / 1,208.00' S= 0.0308 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=0.45 cfs @ 12.80 hrs HW=1,210.33' (Free Discharge)
1=Culvert (Inlet Controls 0.45 cfs @ 1.96 fps)

Pond NWP1: NW Ponding Area 1



Summary for Pond NWP2: NW Ponding Area 2

Inflow Area = 3.899 ac, 23.82% Impervious, Inflow Depth = 3.06" for 25-yr, 24-hr event
Inflow = 15.99 cfs @ 11.90 hrs, Volume= 0.995 af
Outflow = 3.64 cfs @ 12.03 hrs, Volume= 0.995 af, Atten= 77%, Lag= 7.8 min
Primary = 3.64 cfs @ 12.03 hrs, Volume= 0.995 af
Routed to Pond SWP : SW Ponding Area

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 1,209.43' @ 12.03 hrs Surf.Area= 12,549 sf Storage= 11,219 cf

Plug-Flow detention time= 42.2 min calculated for 0.995 af (100% of inflow)
Center-of-Mass det. time= 40.6 min (908.4 - 867.8)

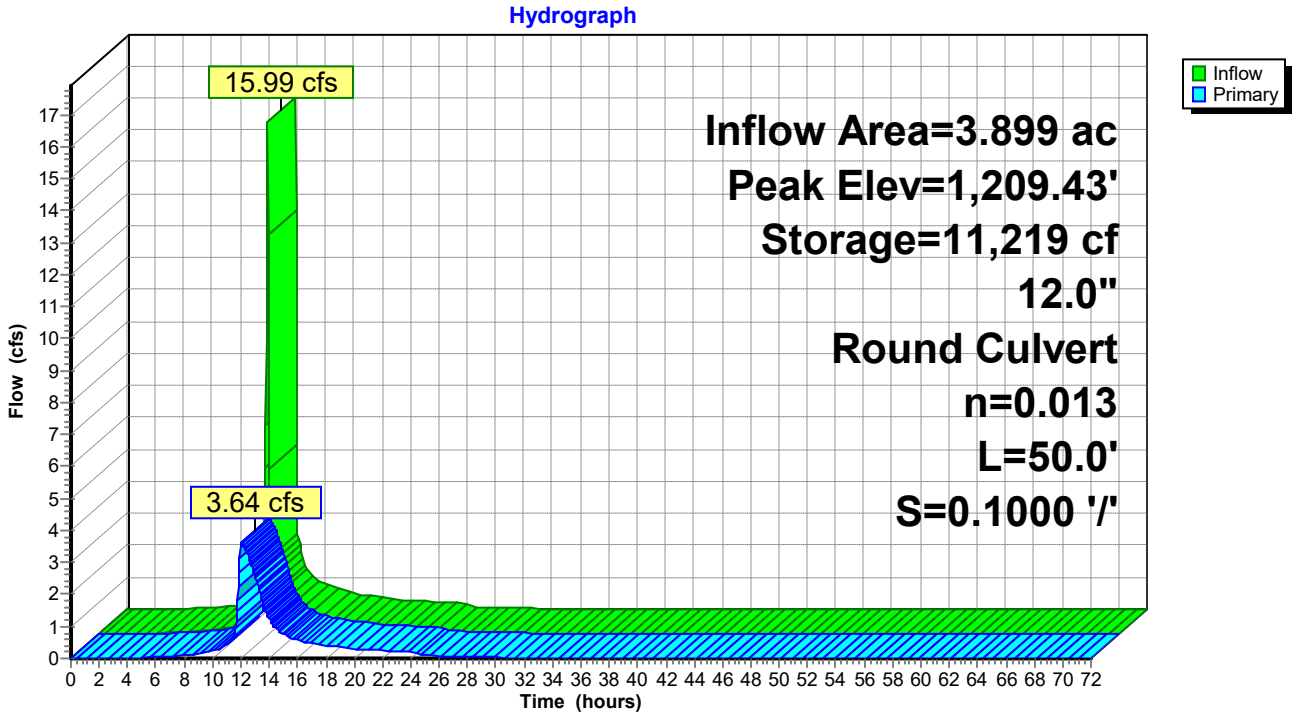
Volume	Invert	Avail.Storage	Storage Description
#1	1,208.00'	18,987 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,208.00	1,315	0	0
1,209.00	11,046	6,181	6,181
1,210.00	14,566	12,806	18,987

Device	Routing	Invert	Outlet Devices
#1	Primary	1,208.00'	**12.0" Round Culvert** L= 50.0' Ke= 0.500 Inlet / Outlet Invert= 1,208.00' / 1,203.00' S= 0.1000 '/' Cc= 0.900 n= 0.013, Flow Area= 0.79 sf

Primary OutFlow Max=3.64 cfs @ 12.03 hrs HW=1,209.42' (Free Discharge)
1=Culvert (Inlet Controls 3.64 cfs @ 4.63 fps)

Pond NWP2: NW Ponding Area 2



Summary for Pond SWP: SW Ponding Area

Inflow Area = 9.945 ac, 22.27% Impervious, Inflow Depth = 3.41" for 25-yr, 24-hr event
Inflow = 41.13 cfs @ 11.90 hrs, Volume= 2.825 af
Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs
Peak Elev= 1,202.81' @ 72.00 hrs Surf.Area= 29,571 sf Storage= 123,047 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

Volume	Invert	Avail.Storage	Storage Description
#1	1,194.00'	342,222 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,194.00	2,520	0	0
1,196.00	6,415	8,935	8,935
1,198.00	11,611	18,026	26,961
1,200.00	18,014	29,625	56,586
1,202.00	25,896	43,910	100,496
1,204.00	34,935	60,831	161,327
1,206.00	44,960	79,895	241,222
1,208.00	56,040	101,000	342,222

Pond SWP: SW Ponding Area

